

GENERAL STRUCTURAL AND CONSTRUCTION NOTES

1.0 GENERAL
1. ALL WORK SHALL CONFORM TO THE '2018 INTERNATIONAL BUILDING CODE' AND TO ALL OTHER APPLICABLE FEDERAL, STATE, AND LOCAL REGULATIONS.
2. ALL CODES AND STANDARDS REFERENCED IN THESE NOTES, INCLUDING SPECIFICATIONS REFERENCED WITHIN AND UNDER FEDERAL, STATE, AND LOCAL REGULATIONS APPLY TO THE DESIGN, CONSTRUCTION, DEMOLITION, QUALITY CONTROL AND SAFETY OF ALL WORK PERFORMED ON THE PROJECT. USE THE LATEST ADOPTED EDITIONS OF THE CODES AND STANDARDS.

2.0 EARTHWORK
1. ENGINEERED (CONTROLLED COMPACTED) FILL WITHIN THE BUILDING AREA SHALL BE CONSTRUCTED PRIOR TO FOOTING EXCAVATION. SEE SPECIFICATIONS FOR REQUIREMENTS OF CONTROLLED COMPACTED FILL.
2. EXCAVATION SHALL BE PERFORMED SO AS NOT TO DISTURB EXISTING ADJACENT BUILDINGS, STREETS, AND UTILITY LINES. VERIFY LOCATION OF ALL UTILITIES PRIOR TO COMMENCEMENT OF WORK. HAND EXCAVATE AROUND UTILITIES AS REQUIRED.

STAMP AREA

3.0 FOUNDATIONS
1. FOUNDATIONS HAVE BEEN DESIGNED FOR AN ALLOWABLE BEARING CAPACITY AND FOOTING ELEVATIONS ESTABLISHED BASED UPON ADJACENT BUILDING INFORMATION, KNOWN INFORMATION FROM ADJACENT SITES, AND SIMILAR SOIL CONDITIONS IN THE PROJECT VICINITY.

4.0 SHEETING AND SHORING
1. SHEETING, SHORING, AND ASSOCIATED EXCAVATION SHALL BE PERFORMED IN ACCORDANCE WITH OSHA GUIDELINES.
2. STRUCTURAL STEEL SOLDIER PILES AND RELATED MISCELLANEOUS FRAMING SHALL HAVE A MINIMUM YIELD STRESS OF 36,000 PSI. WELDING ELECTRODES SHALL BE E70.

5.0 DEMOLITION NOTES
1. REMOVAL AS DESCRIBED HEREIN SHALL BE ACCOMPLISHED WITHOUT STORING ON THE FLOOR EXCESSIVE QUANTITIES OF ANY MATERIALS, RUBBISH, DIRT, DEBRIS, OR WASTE OF ANY SORT RESULTING FROM THE REMOVAL OPERATIONS ON THE FLOOR.
2. ALL DEBRIS SHALL BE REMOVED FROM THE CONSTRUCTION SITE DAILY.
3. THE CONTRACTOR SHALL TAKE PRECAUTIONS TO MAINTAIN FREE PROTECTED ACCESS OF ALL TENANTS, SERVICE PERSONNEL, AND THE PUBLIC THROUGH THE AREAS INVOLVED.

SURVEY REQUIREMENTS
1. BY BIDDING ON THIS PROJECT, THE CONTRACTOR ACCEPTS RESPONSIBILITY FOR THE SURVEY REQUIREMENTS AS SHOWN ON THE CONTRACT DOCUMENTS.
2. A BID SUBMISSION THAT DOES NOT INCLUDE THE REQUIRED SURVEY REQUIREMENTS WILL RESULT IN THE REJECTION OF ANY AND ALL CONSTRUCTION PHASE SUBMISSIONS, RFIS AND SHOP DRAWINGS.

DELEGATED DESIGN / DEFERRED SUBMITTALS
1. ALL DESIGN REQUIREMENTS, LOADING, PERFORMANCE CRITERIA, SUBMISSION STANDARDS AND ANY OTHER APPLICABLE INFORMATION IS LOCATED IN THE GENERAL NOTES, DESIGN DATA, PLANS, SECTIONS, DETAILS AND SPECIFICATIONS (CONSTRUCTION DOCUMENTS) FOR THE DELEGATED DESIGN OF THE COMPONENTS NOTED. BY BIDDING ON THIS PROJECT, THE CONTRACTOR ACCEPTS RESPONSIBILITY FOR THE DESIGN OF THE COMPONENTS DELEGATED BY THESE CONTRACT DOCUMENTS AND ACCEPTS THAT THERE IS ADEQUATE INFORMATION SHOWN ON THE CONTRACT DOCUMENTS TO PERFORM THE DELEGATED DESIGN.

DELEGATED DESIGNS/DEFERRED SUBMITTALS
A. THE MEP CONTRACTOR SHALL PROVIDE PRE-FABRICATED METAL OR WOOD ROOF CURBS, INCLUDING ANCHORAGE, BELOW ROOF TOP EQUIPMENT, WHERE EQUIPMENT SITS ON STEEL DUNNAGE. ALL ATTACHMENTS BETWEEN THE EQUIPMENT AND THE DUNNAGE SHALL BE PROVIDED BY THE MEP CONTRACTOR. ATTACHMENTS SHALL BE DESIGNED TO SUPPORT THE WEIGHT OF THE EQUIPMENT IN ADDITION TO ALL APPLICABLE LATERAL FORCES. REFER TO TYPICAL DETAILS FOR ADDITIONAL INFORMATION.

7.0 CAST-IN-PLACE CONCRETE
1. CONCRETE SHALL BE DESIGNED AND DETAILED IN ACCORDANCE WITH THE BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI-318), AND CONSTRUCTED IN ACCORDANCE WITH THE CRSI MANUAL OF STANDARD PRACTICE.
2. CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE 28-DAY STRENGTH OF 4,000 PSI. AIR ENTRAINMENT 4% TO 6% IN ALL EXPOSED CONCRETE WORK. MAXIMUM WATER/CEMENT RATIO OF 0.45.

Table with 2 columns: Description and Dimension. Rows include Reinforcing Steel in Concrete Cast Against Soil (3"), Reinforcing Steel in Concrete Exposed to Soil or Weather (#5 Bars and Smaller: 1 1/2", #6 Bars and Larger: 2"), Slab and Wall Reinforcing Not Exposed to Soil or Weather (34"), Beam Stirrups, Column Ties, and Horizontal Reinforcing in Shear Walls (1 1/2").

8. SUBMIT TO ARCHITECT/ENGINEER REINFORCING STEEL SHOP DRAWINGS FOR APPROVAL AND MIX DESIGNS FOR REVIEW PRIOR TO PLACING ANY CONCRETE.
A. REINFORCING STEEL PLACING DRAWINGS SHALL BE PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF ACI 314-92, 'DETAILS AND DETAILING OF CONCRETE REINFORCING'. THE PLACING DRAWINGS SHALL SHOW ALL INFORMATION NECESSARY TO FABRICATE AND PLACE THE REINFORCING STEEL.
B. THE SPACING OF ALL REINFORCING STEEL MUST BE COMPUTED BY THE REINFORCING STEEL DETAILER AND MUST BE INDICATED ON THE PLACING DRAWINGS. EXTENT ARROWS MUST BE USED TO CLEARLY INDICATE THE LOCATIONS WHERE GROUPS OF REINFORCING BARS ARE TO BE INSTALLED.

Table with 3 columns: Description, Symbol, Value. Sections include Lateral Load Design (Wind and Seismic), Floor Design Loads, and Component and Cladding (Ult) Wind Pressure (psf).

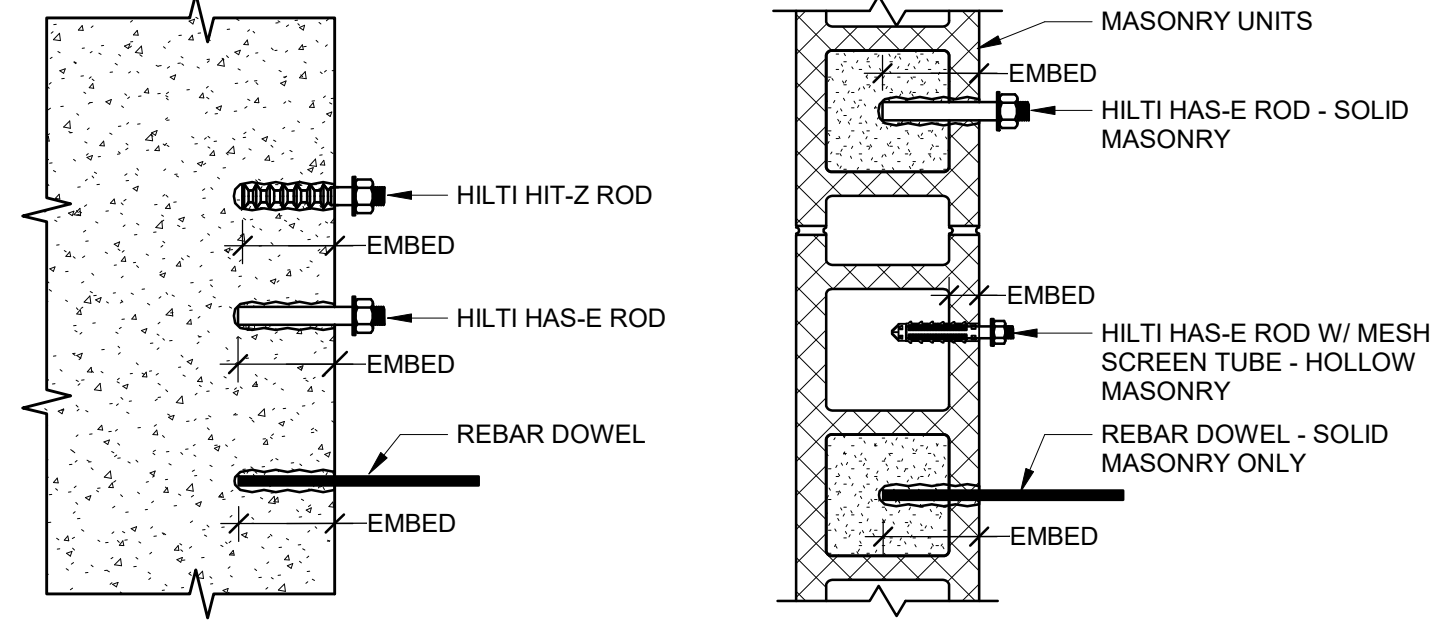
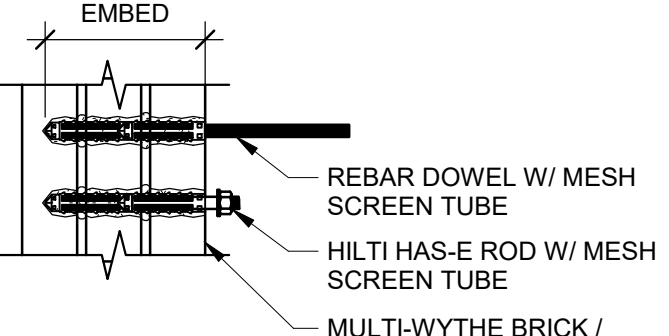
Table with 3 columns: Zone, Effective Wind Area (sf), Wall Neg, Wall Pos. Includes sub-tables for Roof Effective Wind Area and Wall Effective Wind Area. Includes diagrams showing wind directions around a building.

REVISIONS table, Kelly M Aiello Architects logo and contact info, LandscAPE ARCHITECT logo and contact info, LEED CONSULTANT: Verde Architecture Consulting, CITY OF PHILADELPHIA REBUILD / PHILADELPHIA PARKS AND RECREATION logo and address, Kingnesing Recreation Center Building and Site Improvements - Package 2 logo, Pennoni Associates Inc. logo and contact info, Structural General Notes title block with project details and date 08/28/23.



- 12.0 MASONRY
- MASONRY HAS BEEN DESIGNED IN ACCORDANCE WITH THE BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530/ASCE 5) AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE SPECIFICATIONS FOR MASONRY STRUCTURES (ACI 530/ASCE 5), EXCEPT WHERE OTHERWISE MODIFIED BY THESE GENERAL NOTES AND SPECIFICATIONS.
  - MORTAR SHALL CONFORM TO ASTM C270, TYPE M OR S. ALL PORTLAND CEMENT SHALL CONFORM TO ASTM C150, TYPE I. LIME SHALL CONFORM TO ASTM C207 AND MASONRY CEMENT SHALL CONFORM TO ASTM C91.
  - GROUT SHALL CONFORM TO ASTM C476 AND SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI. SLUMP OF GROUT SHALL BE 10 TO 10 INCHES AND THE MAXIMUM AGGREGATE SIZE SHALL BE 3/8" (AGGREGATE GRADED TO PRODUCE FINE GROUT IN CONFORMANCE WITH ASTM C476 AND C404).
  - SOLID AND HOLLOW LOAD BEARING UNITS PER ASTM C90, TYPE N-1, AS REQUIRED TO PROVIDE MINIMUM 28-DAY COMPRESSIVE STRENGTH OF MASONRY, FM = 1900 PSI, UNLESS NOTED OTHERWISE.
  - FULL BED AND HEAD JOINTS SHALL BE PROVIDED.
  - HORIZONTAL JOINT REINFORCING ASTM A62, 9-GAGE TRUSS-TYPE, GALVANIZED.
  - DEFORMED BAR REINFORCEMENT SHALL CONFORM TO ASTM A615, GRADE 60 AND SHALL BE FULL HEIGHT OF WALLS UNLESS OTHERWISE NOTED. PROVIDE BAR SPACERS AND POSITIONERS AS REQUIRED TO PROPERLY LOCATE AND STABILIZE REINFORCING DURING GROUTING OPERATIONS. GROUT ALL REINFORCED CELLS SOLID WITH GROUT.
  - BUILD AND TEST MASONRY PRISMS DURING CONSTRUCTION TO VERIFY FM FOR EACH CLASS OF MASONRY CONSTRUCTION. PRISM TESTS SHALL BE IN ACCORDANCE WITH ASTM E447, METHOD B. HOLLOW CONCRETE UNITS BELOW GRADE AND SLAB ON GRADE SHALL BE NORMAL WEIGHT AND HAVE ALL CELLS GROUTED SOLID.
  - PROVIDE AND INSTALL TEMPORARY BRACING REQUIRED INSURING STABILITY OF ALL WALLS DURING CONSTRUCTION AND UNTIL ERECTION OF ATTACHED STRUCTURAL FRAMING IS COMPLETED.
  - PROVIDE GALVANIZED HORIZONTAL JOINT REINFORCEMENT IN ALL WALLS AND PARTITIONS AT 16" O.C. UNLESS OTHERWISE SHOWN OR NOTED. PROVIDE ONE (1) PIECE PREFABRICATED UNITS AT 8" O.C. AT ALL WALL CORNERS AND INTERSECTIONS.
  - LAP SPLICES FOR DEFORMED REINFORCING BARS USED IN MASONRY CONSTRUCTION SHALL BE 50 BAR DIAMETERS.
  - SUBMIT GROUT MIX DESIGN AND MASONRY UNIT CERTIFICATIONS TO THE ARCHITECT FOR REVIEW. GROUT PLACEMENT SHALL NOT START UNTIL THE PLACEMENT OF REINFORCING HAS BEEN APPROVED BY THE INSPECTION AGENCY.
  - FILL ALL CELLS IN TOP TWO COURSES BELOW FINISHED FLOOR, CMU LINTELS, BOND BEAMS, AND BEAM BEARINGS AND CELLS WITH REINFORCEMENT FULL HEIGHT SOLID WITH GROUT.
  - ALLOW GROUT IN REINFORCED CMU WALLS TO CURE A MINIMUM OF 48 HOURS BEFORE IMPOSING CONCENTRATED OR OTHER LOADS FROM ABOVE.
  - PROVIDE MASONRY ANCHORS SET ON COURSEING AND ATTACHED TO ALL BEAMS AT 32" O.C. HORIZONTAL. COLUMNS AT 24" O.C. VERTICAL. PARTITIONS AND WALLS AT 16" O.C. AT ALL BEAMS, COLUMNS, PARTITIONS AND WALLS ABUTTING OR EMBEDDED IN MASONRY UNLESS NOTED OTHERWISE ON ARCHITECTURAL AND STRUCTURAL DRAWINGS.
  - PROVIDE BOND BEAMS WITH TWO (2) #4 HORIZONTAL REINFORCEMENT CONTINUOUS IN ALL MASONRY WALLS AT EACH FRAMING LEVEL. PROVIDE A MINIMUM OF TWO (2) #4 BARS AT THE ENDS OF ALL WALLS AND ON EACH SIDE OF EACH OPENING.
  - ALL PIERS AND PARTITIONS SHALL BE BONDED OR ANCHORED TO ADJACENT MASONRY WALLS. PROVIDE TIES TO ADJACENT FLOOR AND ROOF CONSTRUCTION IN ACCORDANCE WITH DETAILS ON DRAWINGS.
  - THE CONTRACTOR SHALL VERIFY ALL OPENINGS BELOW LINTELS INDICATED ARE ADEQUATE TO ACCEPT DOORFRAMES, LOUVERS, ETC. AS SHOWN ON THE ARCHITECTURAL AND MECHANICAL DRAWINGS. NOTIFY THE ARCHITECT AND STRUCTURAL ENGINEER OF ANY DISCREPANCIES PRIOR TO LINTEL INSTALLATION.
  - NO OPENINGS SHALL BE PLACED ABOVE ANY LINTEL WITHIN A HEIGHT LESS THAN OR EQUAL TO THE WIDTH OF THE CLEAR OPENING BELOW THE LINTEL, UNLESS SPECIFICALLY SHOWN OR APPROVED BY THE STRUCTURAL ENGINEER.
  - ALL MASONRY WORK TO BE EXECUTED IN COLD WEATHER SHALL BE IN CONFORMANCE WITH THE RECOMMENDATIONS FOR COLD WEATHER CONSTRUCTION FOUND IN THE BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530/ASCE 5-05) AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE SPECIFICATIONS FOR MASONRY STRUCTURES (ACI 530-1.05/ASCE 6-05) WITH THE FOLLOWING ADDITIONS: FOR ALL CONDITIONS WHEN TEMPERATURES FALL BELOW 40 DEGREES F, THE TEMPERATURE OF THE NEWLY LAID MASONRY OR NEWLY GROUTED MASONRY SHALL BE MAINTAINED ABOVE 32 DEGREES F FOR A MINIMUM OF 24 HOURS USING THE METHODS DESCRIBED IN ACI 530.1.
  - THE TESTING AND INSPECTION AGENCY SHALL MONITOR THE PROPORTIONING, MIXING, AND CONSISTENCY OF MORTAR AND GROUT; THE PLACEMENT OF MORTAR, GROUT, AND MASONRY UNITS; AND THE PLACEMENT OF REINFORCING STEEL FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL PREPARE ONE (1) SET OF PRISMS FOR TESTING AT SEVEN (7) DAYS AND ONE (1) SET FOR TESTING AT 28 DAYS. TESTS ARE TO BE CONDUCTED BY THE INSPECTION AND TESTING AGENCY FOR EACH 5,000 SQUARE FEET OF WALL INSTALLED, BUT NOT LESS THAN TWO (2) TESTS. ALL WALL SECTIONS AND PIERS LESS THAN TWO SQUARE FEET IN CROSS-SECTIONAL AREA SHALL BE FULLY GROUTED. PROVIDE VERTICAL MASONRY CONTROL JOINTS AT MAXIMUM 25'-0" ON CENTER UNLESS DETAILED ON ARCHITECTURAL DRAWINGS, COORDINATE LOCATIONS WITH ARCHITECT.

- 11.0 CONCRETE & MASONRY ANCHORAGE
- CAST-IN-PLACE CONCRETE ANCHORS**
- ALL HEADED CONCRETE ANCHORS SHALL BE MANUFACTURED FROM MATERIAL WHICH CONFORMS TO ASTM A108 CARBON STEEL.
  - ALL WELDS SHALL BE MADE IN ACCORDANCE WITH THE STRUCTURAL WELDING CODE, ANSII/A1.1, LATEST EDITION AND WITH THE RECOMMENDATIONS OF THE STEEL MANUFACTURER.
  - THE SPACING, MINIMUM EMBEDMENT, AND INSTALLATION OF THE ANCHORS SHALL BE IN ACCORDANCE WITH ACI 318 AND THE MANUFACTURER'S RECOMMENDED PROCEDURES.
  - STUD ANCHORS SHALL CONFORM TO ASTM A108 AND THE NUTS SHALL CONFORM TO ASTM A563.
- CAST-IN-PLACE MASONRY ANCHORS**
- ALL CAST-IN-PLACE HEADED AND BENT BAR ANCHOR BOLTS INSTALLED IN MASONRY UNITS SHALL BE IN ACCORDANCE WITH TMS402/ACI308 & TMS602/ACI301, LATEST EDITION.
  - HEADED AND BENT BAR ANCHOR BOLTS SHALL BE EMBEDDED IN GROUT. ANCHOR BOLTS OF 1/4-INCH DIAMETER OR LESS ARE PERMITTED TO BE PLACED IN GROUT OR MORTAR BED JOINTS THAT ARE AT LEAST 1/2-INCH THICK.
  - ANCHOR BOLTS PLACED IN THE TOP OF GROUTED CELLS AND BOND BEAMS SHALL MAINTAIN A CLEAR DISTANCE BETWEEN THE BOLT AND THE FACE OF THE MASONRY UNIT OF AT LEAST 1/4-INCH WHEN USING FINE GROUT, AND 1/2-INCH WHEN USING COARSE GROUT.
  - ANCHOR BOLTS PLACED IN DRILLED HOLES IN THE FACE SHELLS OF A HOLLOW MASONRY UNITS SHALL BE PERMITTED TO CONTACT THE MASONRY UNIT WHERE THE BOLT PASSES THROUGH THE FACE SHELL, BUT THE PORTION OF THE BOLT THAT IS WITHIN THE GROUTED CELL SHALL BE POSITIONED TO MAINTAIN A CLEAR DISTANCE BETWEEN THE HEAD OR BENT LEG OF EACH BOLT AND THE MASONRY UNIT OF AT LEAST 1/4-INCH WHEN USING FINE GROUT, 1/2-INCH WHEN USING COARSE GROUT.
  - PLACE ANCHOR BOLTS WITH A CLEAR DISTANCE BETWEEN PARALLEL ANCHOR BOLTS NOT LESS THAN THE NOMINAL DIAMETER OF THE ANCHOR BOLT, NOR LESS THAN 1-INCH.
  - VENEER ANCHORS SHALL INCLUDE CORRUGATED SHEET-METAL ANCHORS, SHEET-METAL ANCHORS, AND WIRE ANCHORS INSTALLED AS FOLLOWS:
    - EMBED ANCHORS IN MORTAR JOINTS AND EXTEND INTO VENEER A MINIMUM OF 1-1/2 INCH, WITH AT LEAST 5/8-INCH MORTAR COVER TO THE OUTSIDE FACE.
    - INSTALL ADJUSTABLE TWO-PIECE ANCHORS IN ACCORDANCE WITH TMS602/ACI301, ARTICLE 3.4.C.
    - PROVIDE AT LEAST ONE (1) ADJUSTABLE TWO-PIECE ANCHOR, ANCHOR OF WIRE SIZE W 1.7, OR 22 GAGE CORRUGATED SHEET METAL ANCHOR 1/8" BY 7" FT. OF WALL AREA. PROVIDE AT LEAST ONE (1) ANCHOR OF OTHER TYPE FOR EACH 3.5 FT. OF WALL AREA.
    - SPACE ANCHORS AT A MAXIMUM OF 32-INCH O.C. HORIZONTALLY AND 24-INCH O.C. VERTICALLY. NOT TO EXCEED REQUIREMENTS OF NOTE 6D ABOVE.
    - PROVIDE ADDITIONAL ANCHORS AROUND OPENINGS LARGER THAN 16-INCH IN EITHER DIRECTION. SPACE ANCHORS AROUND PERIMETER OF OPENING AT A MAXIMUM OF 3-FEET O.C. AND WITHIN 12-INCH OF THE OPENING.
- POST-INSTALLED ANCHORS**
- ALL POST-INSTALLED AND SPECIALTY ANCHORS, INSTALLATION, AND INSPECTIONS SHALL BE IN ACCORDANCE WITH ALL GOVERNING LOCAL MUNICIPAL REGULATIONS, ACI 318, IBC, RELEVANT ICC-ESR REPORTS AND ALL ANCHORS SHALL BE PREQUALIFIED PER ACI 305 TESTING.
  - ALL MECHANICAL AND EPOXY POST INSTALLED ANCHORS (IN CONCRETE OR MASONRY) ARE TO BE INSTALLED IN STRICT CONFORMANCE WITH THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTION (MPI) AS INCLUDED IN THE ANCHOR PACKAGING AND THE APPLICABLE ICC-ESR REPORT INCLUDING, BUT NOT LIMITED TO, DRILL BIT TYPE AND SIZE, PROPER CLEANING AND HOLE PREPARATION, GROUTING EMBEDMENT DEPTHS, CONCRETE TEMPERATURE RANGES, CONCRETE AGE, MOISTURE CONDITION, ETC.
  - ANCHOR CAPACITY USED IN DESIGN SHALL BE BASED ON THE TECHNICAL DATA PUBLISHED BY HILTI OR SUCH OTHER METHOD AS APPROVED BY THE EOR. SUBSTITUTION REQUEST FOR ALTERNATE PRODUCTS MUST BE APPROVED IN WRITING BY THE EOR PRIOR TO USE. THE CONTRACTOR SHALL PROVIDE SIGNED AND SEALED CALCULATIONS, FROM A PROFESSIONAL ENGINEER REGISTERED IN THE LOCAL JURISDICTION, DEMONSTRATING THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERFORMANCE VALUES OF THE SPECIFIED PRODUCT. SUBSTITUTIONS WILL BE EVALUATED BY THEIR HAVING AN ICC ESR SHOWING COMPLIANCE WITH THE RELEVANT BUILDING CODE FOR SEISMIC USES, LOAD RESISTANCE, INSTALLATION CATEGORY AND AVAILABILITY OF COMPREHENSIVE INSTALLATION INSTRUCTIONS. ADHESIVE ANCHOR EVALUATION WILL ALSO CONSIDER CREEP, IN-SERVICE TEMPERATURE AND INSTALLATION TEMPERATURE.
  - ALL ANCHORS SHALL BE INSTALLED IN STRICT CONFORMANCE WITH THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTION (MPI) AS INCLUDED IN THE ANCHOR PACKAGING AND THE APPLICABLE ICC-ESR REPORT, INCLUDING, BUT NOT LIMITED TO, DRILL BIT TYPE AND SIZE, PROPER CLEANING AND HOLE PREPARATION, GROUTING EMBEDMENT DEPTHS, CONCRETE TEMPERATURE RANGES, CONCRETE AGE, MOISTURE CONDITION, ETC.
  - EXISTING REINFORCING BARS IN THE CONCRETE OR MASONRY MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS INDICATED ON THE STRUCTURAL DRAWINGS. UNLESS NOTED OTHERWISE, THE REINFORCING BARS MAY NOT BE CUT. THE CONTRACTOR SHALL REVIEW THE EXISTING STRUCTURAL DRAWINGS (IF AVAILABLE) AND SHALL TAKE STEPS TO LOCATE THE POSITION OF THE REINFORCING BARS AT THE LOCATIONS OF THE CONCRETE ANCHORS USING NON-DESTRUCTIVE TESTING (FERROSCAN, GPR, XRAY, OR OTHER APPROVED METHOD).
  - DRILL AND GROUT/EPOXY REINFORCING BAR DOWELS AS SHOWN ON THE PLANS, DETAILS AND AS APPROVED, UNLESS NOTED OTHERWISE. EMBED BARS AS REQUIRED TO DEVELOP THE FULL TENSION CAPACITY OF THE BARS.
  - EXCEPT WHERE INDICATED ON THE DRAWINGS, THE FOLLOWING POST-INSTALLED ANCHORS ARE APPROVED AS PROVIDED BY HILTI, INC. SUBSTITUTION OF THESE ANCHORS AND/OR USE OF ANY OTHER SPECIALTY ANCHORS SHALL BE SUBMITTED TO THE EOR FOR APPROVAL.



**CONCRETE ANCHORS MASONRY ANCHORS**

**NOTES:**

- DRILL & EPOXY ANCHORS IN ACCORDANCE WITH THE MPI.
  - UTILIZE HILTI HIT-HY 200 IN CRACKED AND UNCRACKED CONCRETE CONSTRUCTION UNLESS NOTED OTHERWISE ON PLANS AND DETAILS. HILTI RE-500 V3 IS AN ACCEPTABLE SUBSTITUTION.
  - UTILIZE HILTI HIT-HY 270 IN MASONRY AND MULTI-WYTHE CONSTRUCTION. PROVIDE APPROPRIATELY SIZED SCREEN TUBES IN HOLLOW AND MULTI-WYTHE MASONRY CONSTRUCTION ONLY.
- REFER TO PLANS AND DETAILS FOR QUANTITY, ANCHOR TYPE, DIAMETER, AND MINIMUM EMBEDMENT DEPTH.
- REFER TO THE MPI FOR INFORMATION NOT PROVIDED, INCLUDING BUT NOT LIMITED TO, MINIMUM EDGE DISTANCE, MINIMUM ANCHOR SPACING, CLEANING PROCEDURES, AND INSTALLATION TORQUE REQUIREMENTS BASED ON THE SELECTED ANCHOR TYPE, DIAMETER, APPLICABLE EPOXY SERIES AND CONSTRUCTION TYPE.

SUBSTRATE	MECHANICAL ANCHORS	
	ANCHOR TYPE	APPROVED SPECIFIED ANCHOR
CRACKED & UNCRACKED CONCRETE	SCREW ANCHOR	HILTI KWIK HUS E2 AND E2-1 PER ICC ESR-3027
		HILTI KWIK BOLT-TZ PER ICC ESR-1917
	EXPANSION ANCHOR	HILTI KWIK BOLT 3 PER ICC ESR-2302 (UNCRACKED CONCRETE ONLY)
		HILTI HDA UNDERCUT ANCHORS PER ICC ESR-1546 HILTI HSL-3 PER ICC ESR-1545
SOLID GROUTED MASONRY	EXPANSION ANCHOR	HILTI KWIK BOLT 3 PER ICC ESR-1395
SUBSTRATE	ADHESIVE ANCHORS	
	APPROVED SPECIFIED ADHESIVE & ANCHOR	
CRACKED & UNCRACKED CONCRETE	HILTI HIT-HY 200 SAFE SET SYSTEM WITH HAS-E ROD, HIT-Z ROD, OR REBAR PER ICC ESR-3187	
	HILTI HIT-HY 500 V3 SAFE SET SYSTEM WITH HAS-E ROD, HIT-Z ROD, OR REBAR PER ICC ESR-3814	
MASONRY	HILTI HIT-HY 270 MASONRY ADHESIVE ANCHORING SYSTEM WITH HAS-E ROD OR REBAR PER ICC ESR-4143	

- EPOXY CARTRIDGES SHALL UTILIZE THE CORRECT MIXING NOZZLE AS SUPPLIED BY THE MANUFACTURER. THE CONTRACTOR SHALL NOT RE-USE, MODIFY (CUT) OR REMOVE THE MIXING INSERT FROM THE MIXING NOZZLE.
- THE CONTRACTOR SHALL UTILIZE APPROPRIATELY SIZED MESH SCREEN TUBES IN HOLLOW AND MULTI-WYTHE BRICK AND MASONRY APPLICATIONS.
- ALL EPOXY ANCHORS THAT ARE TO BE INSTALLED HORIZONTALLY OR UPWARDLY INCLINED OVERHEAD ARE TO BE INSTALLED UTILIZING THE HILTI "PISTON-PLUG" ACCESSORY, REGARDLESS OF THE EPOXY MANUFACTURER OR MODEL. USING AN EXTENSION TUBE AND RETAINING CAP IS NOT AN ACCEPTABLE METHOD AND SHALL BE REJECTED BY THE INSPECTOR.
- ALL ANCHORS, INCLUDING WASHERS, AND NUTS SHALL HAVE THE FOLLOWING CORROSION PROTECTIONS, UNLESS NOTED OTHERWISE:
  - INTERIOR USE, NON-CORROSIVE CONDITION - ZINC COATED PER ASTM B633
  - EXTERIOR WEATHER OR IN CONTACT WITH PLT LUMBER OR CORROSIVE INDUCING ELEMENTS - MECHANICALLY DEPOSITED ZINC COATING PER ASTM B695 OR HOT-DIP GALVANIZED (HDG) PER ASTM A153
  - NEAR SALT WATER OR EXTERIOR CORROSIVE ENVIRONMENTS - STAINLESS STEEL AISI 316
- ALL ADHESIVE ANCHORS INSTALLED HORIZONTALLY OR UPWARDLY INCLINED (OVERHEAD) TO SUPPORT SUSTAINED TENSION LOADS SHALL BE PERFORMED BY CERTIFIED PERSONNEL.
- CERTIFICATION SHALL INCLUDE WRITTEN AND PERFORMANCE TESTS IN ACCORDANCE WITH THE ACI/CRS ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM, OR AN APPROVED EQUIVALENT. THE CONTRACTOR SHALL SUBMIT CERTIFICATES FOR RECORD PRIOR TO INSTALLATION OF ANCHORS. THE ACCEPTABILITY OF CERTIFICATION OTHER THAN ACI/CRS ADHESIVE ANCHOR INSTALLER CERTIFICATION SHALL BE THE RESPONSIBILITY OF THE EOR.
- THE ANCHOR MANUFACTURER SHALL MAKE A REPRESENTATIVE AVAILABLE TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL OF THEIR ANCHORING PRODUCTS SPECIFIED ON THE STRUCTURAL DRAWINGS. TRAINING SHALL BE AT THE CONTRACTOR'S REQUEST AND AT NO ADDITIONAL CHARGE TO THE CONTRACTOR OR OWNER. THE ANCHOR MANUFACTURER'S REPRESENTATIVE SHALL BE PRESENT DURING THE INITIAL INSTALLATION OF EACH TYPE OF ANCHOR TO REVIEW AND APPROVE THE CONTRACTOR'S INSTALLATION PROCEDURES.
- THE OWNER'S TESTING AGENCY SHALL OBSERVE THE INITIAL INSTALLATION OF EACH ANCHOR TYPE AND DURING CONSTRUCTION AT INTERVALS IN ACCORDANCE WITH THE IBC CH 17 AND ACI 318. ADHESIVE ANCHORS INSTALLED HORIZONTALLY OR VERTICALLY INCLINED (OVERHEAD) TO SUPPORT SUSTAINED TENSION LOADS SHALL BE CONTINUOUSLY OBSERVED BY THE SPECIAL INSPECTOR. THE SPECIAL INSPECTOR SHALL PROVIDE WRITTEN REPORTS TO THE EOR AND BUILDING OFFICIAL THAT INDICATE THAT THE MATERIALS USED AND THE INSTALLATION PROCEDURES USED CONFORM WITH THE APPROVED COMMENTS AND THE MPI. THE REPORTS SHALL INCLUDE DESCRIPTIONS OF THE MATERIALS AND PROCEDURES USED, INCLUDING BUT NOT LIMITED TO THE FOLLOWING:
  - ANCHOR INSTALLATION ENVIRONMENT (DRY OR SATURATED CONCRETE, CONCRETE TEMPERATURE RANGE)
  - DESCRIPTION OF THE DRILLING METHOD
  - DESCRIPTION OF THE CLEANING PROCEDURE FOR THE SELECTED ANCHOR TYPE
  - DESCRIPTION OF THE SELECTED ANCHOR TYPE AND SIZE RANGE (THREADED ROD OR REINFORCING BAR)

**STRUCTURAL STEEL**

- FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO THE "STEEL CONSTRUCTION MANUAL," AMERICAN INSTITUTE OF STEEL CONSTRUCTION INCLUDING SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS, SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS, AND AISC CODE OF STANDARD PRACTICE.
- ALL WELDING SHALL BE PERFORMED BY CERTIFIED WELDERS AND SHALL CONFORM TO "STRUCTURAL WELDING CODE ANSII/A1.1," AMERICAN WELDING SOCIETY.

SHAPE	SPECIFICATION
WIDE FLANGE SHAPES:	ASTM A992 OR A572, GRADE 50.
STRUCTURAL SHAPES AND PLATES:	ASTM A36, A572 OR A992.
STEEL PIPE:	ASTM A53, GRADE B.
STEEL TUBING (SQUARE, RECT. OR ROUND):	ASTM A500, GRADE C.
GALVANIZED STRUCTURAL STEEL:	
STRUCTURAL SHAPES AND RODS BOLTS, FASTENERS AND HARDWARE	ASTM A123 ASTM A153.
STAINLESS STEEL (F <sub>y</sub> = 40 KSI):	
STRUCTURAL BARS, ROUNDS AND HOT ROLLED SHAPES	ASTM A276.
HIGH STRENGTH BOLTING MATERIAL	ASTM A193. ASTM A194.
HIGH STRENGTH NUTS	
ANCHOR RODS	ASTM F1554, GRADE 36, UNLESS NOTED OTHERWISE.
BOLTED CONNECTION	ASTM A325 HIGH STRENGTH BOLTS 3/4" MINIMUM DIAMETER, UNLESS NOTED OTHERWISE.
WELDING ELECTRODES (MINIMUM WELD SIZE SHALL BE 3/16" UNLESS NOTED OTHERWISE)	E70XX (FOR MANUAL ARC WELDING)

**CONNECTIONS:**

- BOLTED CONNECTIONS SHALL BE WITH ASTM A325 HIGH STRENGTH BOLTS 3/4" MINIMUM DIAMETER, UNLESS NOTED OTHERWISE. ALL BOLTS SHALL BE INSTALLED SNUG TIGHT UNLESS NOTED OTHERWISE IN CONTRACT DOCUMENTS OR AISC.
  - ALL CONNECTIONS SHALL BE SYMMETRICAL ABOUT THE AXIS OF THE MEMBER CONNECTED. PROVIDE ONLY ONE GRADE OF BOLT FOR EACH BOLT DIAMETER TO BE USED IN THE CONNECTIONS. DO NOT MIX GRADE OF BOLTS.
  - ALL CONNECTIONS SHALL BE FRAMED BEAM CONNECTIONS' DESIGNED IN ACCORDANCE WITH THE AISC MANUAL FOR THE REACTIONS SHOWN ON PLAN, BUT NOT LESS THAN 12 KIIPS. PROVIDE CONNECTIONS FULL DEPTH OF SUPPORTING BEAM, UNLESS OTHERWISE APPROVED. MINIMUM TWO (2) BOLTS PER CONNECTION.
  - PROVIDE SIGNED AND SEALED CALCULATIONS FOR ALL CONNECTION DESIGNS NOT INDICATED ON THE DRAWINGS. THE FABRICATOR IS RESPONSIBLE FOR THE SELECTION, DESIGN, AND DETAILING OF ALL CONNECTIONS NOT FULLY DETAILED IN THE CONTRACT DOCUMENTS. TYPICAL CONNECTION DETAILS HAVE BEEN INDICATED ON THE DRAWINGS FOR DESIGN INTENT ONLY. THE FABRICATOR SHALL HAVE A REGISTERED PROFESSIONAL ENGINEER LICENSED IN PROJECT'S JURISDICTION PREPARE AND/OR REVIEW THE CONNECTION DESIGN PRIOR TO SUBMITTING THE SHOP DRAWINGS TO THE ARCHITECT/ENGINEER FOR REVIEW AND APPROVAL. THE INITIAL SHOP DRAWINGS SUBMITTAL SHALL INCLUDE PROPOSED CONNECTION DETAILS AND JOB STANDARDS. CALCULATIONS SHALL SHOW COMPLIANCE WITH ALL CLEAR DIMENSIONS, CONNECTIONS. SHOP DRAWINGS SHALL DIRECTLY REFERENCE CONNECTION DETAILS ON SUBMITTAL.
- CUTS, HOLES, CORING, ETC. REQUIRED FOR OTHER TRADES OR FIELD CONDITIONS SHALL BE SHOWN ON THE SHOP DRAWINGS AND MADE IN THE SHOP. CUTTING OR BURNING OF MAIN STRUCTURAL MEMBERS IN THE FIELD WILL NOT BE PERMITTED.
  - SUBMIT SHOP DRAWINGS FOR FABRICATION AND ERECTION OF STRUCTURAL STEEL. CLEARLY INDICATE COORDINATED DIMENSIONS OF MECHANICAL UNIT AND ROOF PENETRATION SIZES. SHOP AND ERECTION DRAWINGS MUST SHOW ALL SHOP/FLOOR AND FIELD WELDS. INITIAL SHOP DRAWING SUBMITTAL SHALL INCLUDE PROPOSED CONNECTION DETAILS AND JOB STANDARDS. PROVIDE SIGNED AND SEALED CALCULATIONS FOR ALL CONNECTION DESIGNS NOT INDICATED ON THE DRAWINGS. CALCULATIONS SHALL SHOW DESIGN CAPACITIES FOR ALL CONNECTIONS.
  - STEEL MEMBERS SHOWN ON PLAN SHALL BE EQUALLY SPACED UNLESS NOTED OTHERWISE.
  - CAMBER INDICATED ON THESE DRAWINGS IS THE REQUIRED CAMBER AFTER FINAL ERECTION AND INCLUDES ALL MILL TOLERANCES.
  - THE GENERAL CONTRACTOR AND STEEL ERECTOR SHALL NOTIFY THE STRUCTURAL ENGINEER OF ANY FABRICATION OR ERECTION ERRORS OR DEVIATIONS AND RECEIVE WRITTEN APPROVAL BEFORE ANY FIELD CORRECTIONS ARE MADE.
  - WELDS TO THE EXISTING MILL WILL NOT BE ALLOWED AND THE CONTRACTOR SHALL ANTICIPATE USING FIELD BOLTED CONNECTIONS TO THE EXISTING STEEL.
  - ALL STEEL SHALL BE PAINTED WITH SHOP STANDARD PRIMER UNLESS NOTED OTHERWISE.
  - STEEL ANGLES AND PLATES ALONG WITH BOLTS AND WASHERS, IN DIRECT CONTACT WITH EXTERIOR FINISH MASONRY, AND ALL EXTERIOR EXPOSED STRUCTURAL STEEL, SHALL BE HOT-DIPPED GALVANIZED.
  - ALL EXTERIOR EXPOSED STRUCTURAL STEEL SHALL BE HOT-DIPPED GALVANIZED PER ASTM A123.
  - SPANDRELS AND COLUMNS ADJACENT TO MASONRY SHALL HAVE ADJUSTABLE MASONRY TIES.
  - EXISTING FRAMING REQUIRING WELDING SHALL BE THOROUGHLY CLEANED TO ENSURE PROPER WELDING. PROVIDE TEMPORARY SHORING WHEN WELDING TO EXISTING STEEL.
  - FIELD WELDED SURFACES WITHIN FOUR (4) INCHES OF WELD SHALL BE CLEANED AND GROUND SMOOTH. AFTER WELDING COAT THE EXPOSED AREA WITH APPROPRIATE PRIMER/PAINTS AS SPECIFIED.
  - FIELD WELDED EXPOSED GALVANIZED SURFACES WITHIN FOUR (4) INCHES OF WELD SHALL BE CLEANED AND GROUND SMOOTH. AFTER WELDING COAT THE EXPOSED AREA WITH GALVANIZING REPAIR PAINT. GALVANIZING REPAIR PAINT SHALL BE A HIGH ZINC DUST CONTENT PAINT COMPLYING WITH FEDERAL SPECIFICATIONS DOD-P-21035A OR SSPC-PAINT-20, COLD GALVANIZING COMPOUND BY ZRC PRODUCTS CO. OR EQUAL.
  - VISUALLY INSPECT ALL FILLET WELDS. 10% OF ALL FIELD FILLET WELDS IN PRIMARY CONNECTIONS AND MULTI-PASS WELDS SHALL BE TESTED BY THE MAGNETIC PARTICLE METHOD, COMPLYING WITH E109, PERFORMED ON THE ROOT PASS AND ON THE FINISHED WELD.
  - 100% OF FULL PENETRATION WELDS SHALL HAVE ULTRASONIC INSPECTION, COMPLYING WITH ASTM E164.
  - 100% OF WELDS IN BEAM AND COLUMN MOMENT CONNECTIONS SHALL HAVE ULTRASONIC INSPECTION, COMPLYING WITH ASTM E164.
  - FIELD TEST BOLTED CONNECTIONS AND SHEAR STUDS IN ACCORDANCE WITH AISC.
  - DELETE PAINT ON ALL STEEL TO RECEIVE SPRAYED-ON FIREPROOFINGS OR CONCRETE ENCASEMENT.
  - ALL STEEL SHALL BE THOROUGHLY CLEANED BY POWER TOOL CLEANING PRIOR TO PAINTING. ALL ARCHITECTURALLY EXPOSED STRUCTURAL STEEL SHALL BE CLEANED WITH COMMERCIAL BLAST CLEANING.
  - ALL DISSIMILAR METALS SHALL BE TREATED OR PROPERLY SEPARATED TO PREVENT GALVANIC AND/OR CORROSIVE EFFECTS.

**17.0 COLD FORM METAL FRAMING**

- LIGHT GAGE METAL FRAMING SHALL BE DESIGNED AND DETAILED ACCORDING WITH "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS - 2007 EDITION," AMERICAN IRON AND STEEL INSTITUTE.
- ALL STUD AND/OR JOIST FRAMING MEMBERS SHALL BE OF THE TYPE, SIZE, AND GAGE AS REQUIRED BY DESIGN. SIZE AND GAGE SHALL NOT BE LESS THAN SHOWN ON DRAWINGS.
- LIGHT GAGE METAL FRAMING PROPERTIES ARE BASED ON PRODUCTS MANUFACTURED BY (MARINO WARE, A DIVISION OF WARE INDUSTRIES, INC.) MEMBERS BY OTHER MANUFACTURERS MAY BE SUPPLIED PROVIDED LOAD CARRYING CAPACITY BASED ON MANUFACTURER'S STANDARD LOAD TABLES, AND DETAIL CHARACTERISTICS EQUAL OR EXCEED THOSE OF MATERIALS SPECIFIED AND IF APPROVED BY THE ARCHITECT AND STRUCTURAL ENGINEER.
- ALL GALVANIZED STUDS, JOISTS, TRACK, BRIDGING, AND ACCESSORIES, 12, 14, AND 16 GAGE, SHALL BE FORMED FROM STEEL THAT CORRESPONDS TO THE REQUIREMENTS OF ASTM A583, GRADE 58, WITH A MINIMUM YIELD OF 50,000 PSI.
- ALL GALVANIZED STUDS, JOIST, AND TRACK, BRIDGING AND ACCESSORIES, 18 AND 20 GAGE, SHALL BE FORMED FROM STEEL THAT CORRESPONDS TO THE REQUIREMENTS OF ASTM A583, GRADE 58, WITH A MINIMUM YIELD OF 33,000 PSI.
- ALL STUDS, JOIST, AND ACCESSORIES, SHALL BE FORMED FROM STEEL HAVING A G60 GALVANIZED COATING IN CONFORMANCE WITH ASTM C565.
- PRIOR TO PREFABRICATION OF FRAMING, THE CONTRACTOR SHALL SUBMIT SIGNED AND SEALED FABRICATION AND ERECTION DRAWINGS TO THE ARCHITECT FOR REVIEW. INCLUDE WITH THE DRAWINGS CROSS SECTIONS, PLANS AND/OR ELEVATIONS DEPICTING COMPONENTS TYPES AND LOCATIONS FOR EACH UNIQUE FRAMING APPLICATION, CONNECTION DETAILS DEPICTING FASTENER TYPE, AND QUANTITY. SUBMIT SIGNED AND SEALED CALCULATIONS PREPARED BY AN ENGINEER REGISTERED IN THE PROJECT'S JURISDICTION.
- FRAMING COMPONENTS MAY BE PREASSEMBLED INTO PANELS PRIOR TO ERECTING. PREFABRICATED PANELS SHALL BE SQUARE WITH COMPONENTS ATTACHED IN A MANNER AS TO PREVENT RACKING AND TO MINIMIZE DISTORTION WHILE LIFTING AND TRANSPORTING.
- CUTTING OF STEEL FRAMING SHALL BE BY SAW, SHEAR OR PLASMA CUTTING EQUIPMENT ONLY.
- TEMPORARY BRACING SHALL BE PROVIDED UNTIL ERECTION IS COMPLETE AND ALL ATTACHED ADJACENT FRAMING IS COMPLETE.
- INSULATION SHALL BE PLACED IN COMPONENTS INACCESSIBLE TO THE INSULATION CONTRACTOR AFTER THEIR INSTALLATION.
- SPLICES IN AXIALLY LOADED STUDS ARE NOT PERMITTED.
- WHERE SPLICING OF TRACK IS NECESSARY BETWEEN STUD SPACING, A PIECE OF STUD SHALL BE PLACED BETWEEN ADJACENT TRACKS AND FASTENED BY WELDS OR SCREWS TO EACH SIDE OF THE TRACK, EACH END.
- STUDS SHALL BE PLUMBED, ALIGNED, AND SECURELY ATTACHED TO THE FLANGES OR ENDS OF BOTH UPPER AND LOWER TRACKS.
- AXIALLY LOADED STUDS SHALL BE INSTALLED IN A MANNER WHICH WILL ASSURE THAT ENDS OF THE STUDS ARE POSITIONED AGAINST THE INSIDE TRACK WEB, PRIOR TO STUD AND TRACK ATTACHMENT. STUDS SHALL BE SQUARELY CUT AND POSITIVELY CLAMPED AND POSITIONED UNTIL PROPERLY FASTENED.
- WALL STUD BRIDGING SHALL BE ATTACHED IN A MANNER TO PREVENT STUD ROTATION. BRIDGING, OF THE TYPE AND SPACING SHOWN ON THE CONTRACT OR SHOP DRAWINGS SHALL BE INSTALLED PRIOR TO LOADING. BRIDGING SPACING SHALL BE AS REQUIRED BY DESIGN, BUT SHALL NOT EXCEED 5'-0" ON CENTER.
- PROVISION FOR STRUCTURE VERTICAL MOVEMENT SHALL BE PROVIDED WHERE INDICATED ON THE PLANS USING VERTICAL SLIDE CLIPS OR OTHER MEANS. FRAME BOTH SIDES OF EXPANSION JOINTS WITH SEPARATE STUDS; DO NOT BRIDGE THE EXPANSION JOINTS WITH STUD SYSTEM COMPONENTS. FRAMING WALL OPENINGS SHALL BE PROVIDED WITH STUDS AS SHOWN ON THE PLANS AND SHOP DRAWINGS. PROVIDE ADDITIONAL JACK AND KING STUDS AS REQUIRED AT ALL OPENINGS WHICH EXCEED 24 INCHES.
- JOISTS SHALL BE LOCATED DIRECTLY OVER BEARING STUDS OR A LOAD DISTRIBUTION MEMBER TO BE PROVIDED AT THE TOP TRACK.
- CONNECTIONS SHALL BE BY WELDING, RIVETING, BOLTING OR OTHER APPROVED FASTENING DEVICES OR METHODS PROVIDING POSITIVE ATTACHMENT AND RESISTANCE TO LOOSENING. FASTENERS SHALL BE OF COMPATIBLE MATERIAL.
- WELDED CONNECTIONS SHALL BE PERFORMED IN ACCORDANCE WITH AWS SPECIFICATION FOR WELDING SHEET STEEL IN STRUCTURES, D1.3.
- CONTRACTOR SHALL REFER TO INSTALLATION INSTRUCTIONS PUBLISHED BY THE SCREW MANUFACTURER AND ASTM C564 FOR MINIMUM SPACING AND EDGE DISTANCES REQUIREMENTS AND TORQUE REQUIREMENTS.

**18.0 STRUCTURAL WOOD**

- DESIGN, FABRICATION, AND CONSTRUCTION OF WOOD FRAMING SHALL CONFORM WITH THE FOLLOWING CODES AND STANDARDS:
  - NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION", 2005 EDITION, (WITH SUPPLEMENT), AMERICAN FOREST AND PAPER ASSOCIATION.
  - "TIMBER CONSTRUCTION MANUAL," FOURTH EDITION, AS ADOPTED BY THE AMERICAN INSTITUTE OF WOOD CONSTRUCTION, INCLUDING THE "CODE OF STANDARD PRACTICE," AISC 108.
- BASE DESIGN VALUES FOR ROOF/FLOOR JOIST FRAMING: DOUG-FIR NO. 1 AND NO. 2 (F<sub>y</sub> = 850 PSI, F<sub>v</sub> = 180 PSI, E = 1,800,000 PSI) MINIMUM.
- BASE DESIGN VALUE FOR WOOD STUDS AND BRACING: DOUG FIR STUD MINIMUM COMPRESSION PARALLEL TO GRAIN F<sub>c</sub> = 850 PSI, MINIMUM TENSION PARALLEL TO GRAIN, F<sub>t</sub> = 400 PSI, MINIMUM COMPRESSION PERPENDICULAR TO GRAIN, 625 PSI.
- ALL PLYWOOD SHEATHING SHALL COMPLY WITH APA, PLYWOOD SHALL MEET C-D INTERIOR APA, STRUCTURAL I AND II C-D INTERIOR APA, OR STRUCTURAL I AND II C-C EXTERIOR APA. ATTACHMENT TO BE IN ACCORDANCE WITH IBC REQUIREMENTS. ALL PLYWOOD TO HAVE EXTERIOR GLUE.
  - ROOF SHEATHING SHALL BE APA RATED SHEATHING, 1/2" THICK, 4/20.
  - FLOOR SHEATHING SHALL BE APA RATED STURD-FLOOR, 3/4" THICK, 48/24.
  - WALL SHEATHING SHALL BE APA RATED SHEATHING 1/2" THICK, 32/16.
- PROVIDE NAILING PATTERN IN COMPLIANCE WITH IBC RECOMMENDED FASTENING SCHEDULE WHEN JOINING TWO OR MORE FRAMING MEMBERS.
- PROVIDE DOUBLE LAYER PLYWOOD UNDER ALL CERAMIC TILE FLOORS.
- BASE DESIGN VALUE FOR ALL OTHER STRUCTURAL WOOD FRAMING: MINIMUM EXTREME FIBER IN BENDING, F<sub>b</sub> = 850 PSI; MINIMUM HORIZONTAL SHEAR, F<sub>v</sub> = 180 PSI; MINIMUM COMPRESSION PARALLEL TO GRAIN, F<sub>c</sub> = 1,400 PSI.
- HANGER CONNECTIONS FOR JOISTS, BEAMS, AND MANUFACTURED WOOD FRAMING SHALL BE STRONG-TIE CONNECTORS BY SIMPSON.
- SEE INTERNATIONAL BUILDING CODE FOR MINIMUM BRACING AND FASTENING REQUIREMENTS.
- MEMBERS SHALL BE SET WITH CROWN UP AND HAVE A MINIMUM OF 3" BEARING.
- PROVIDE ADDITIONAL JOIST UNDER PARALLEL NON-LOADING BEARING PARTITIONS THAT RUN MORE THAN 1/3 THE SPAN OF THE JOIST.
- SPLICE DOUBLE SOLE PLATES DIRECTLY OVER STUD. STAGGER SPLICE OF EACH PLATE.
- ALL JOISTS AND RAFTERS SHALL BE RIGIDLY BRIDGED AT INTERVALS NOT EXCEEDING 8'-0".
- ALL BOLTS AND LAG BOLTS SHALL BE FITTED WITH GALVANIZED, MALLEABLE IRON OR STEEL PLATE WASHERS.
- ALL WOOD MEMBERS EXPOSED TO EXTERIOR TO BE PRESSURE TREATED.
- PROVIDE FASTENERS, ANCHORS AND CONNECTORS WITH ADEQUATE CORROSION PROTECTION WHERE IN CONTACT WITH TREATED WOOD. PROVIDE MINIMUM ZMAX COATING WHERE SIMPSON CONNECTORS ARE USED IN CONTACT WITH TREATED WOOD.

**REVISIONS**

ISSUE	DATE	DESCRIPTION
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REVIEWED BY:

PROJECT COORDINATOR

SEALS



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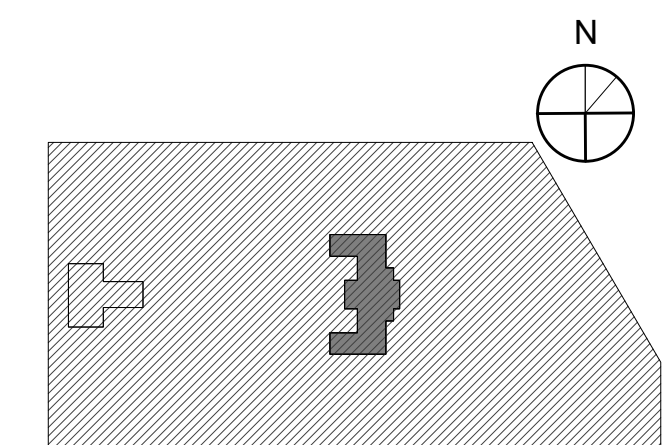
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11TH FLOOR, ONE PARKWAY BUILDING  
PHILADELPHIA PENNSYLVANIA

**KINGSESSING RECREATION CENTER BUILDING AND SITE IMPROVEMENTS - PACKAGE 2**

KEY PLAN



DRAWING TITLE

**STRUCTURAL GENERAL NOTES**

PROJECT NO. **21070** DRAWING NO.

DATE 06/29/23  
SCALE As Indicated  
DRAWN BY WZD  
CHECKED BY fpu

NOTE: ALL DIMENSIONS AND CONDITIONS SHALL BE VERIFIED BY THE CONTRACTOR AT THE SITE BEFORE PROCEEDING WITH THE WORK.

**PACKAGE 2 - IFB**  
**NOT FOR CONSTRUCTION**  
08/28/23

**S002-R.2**

STAMP AREA

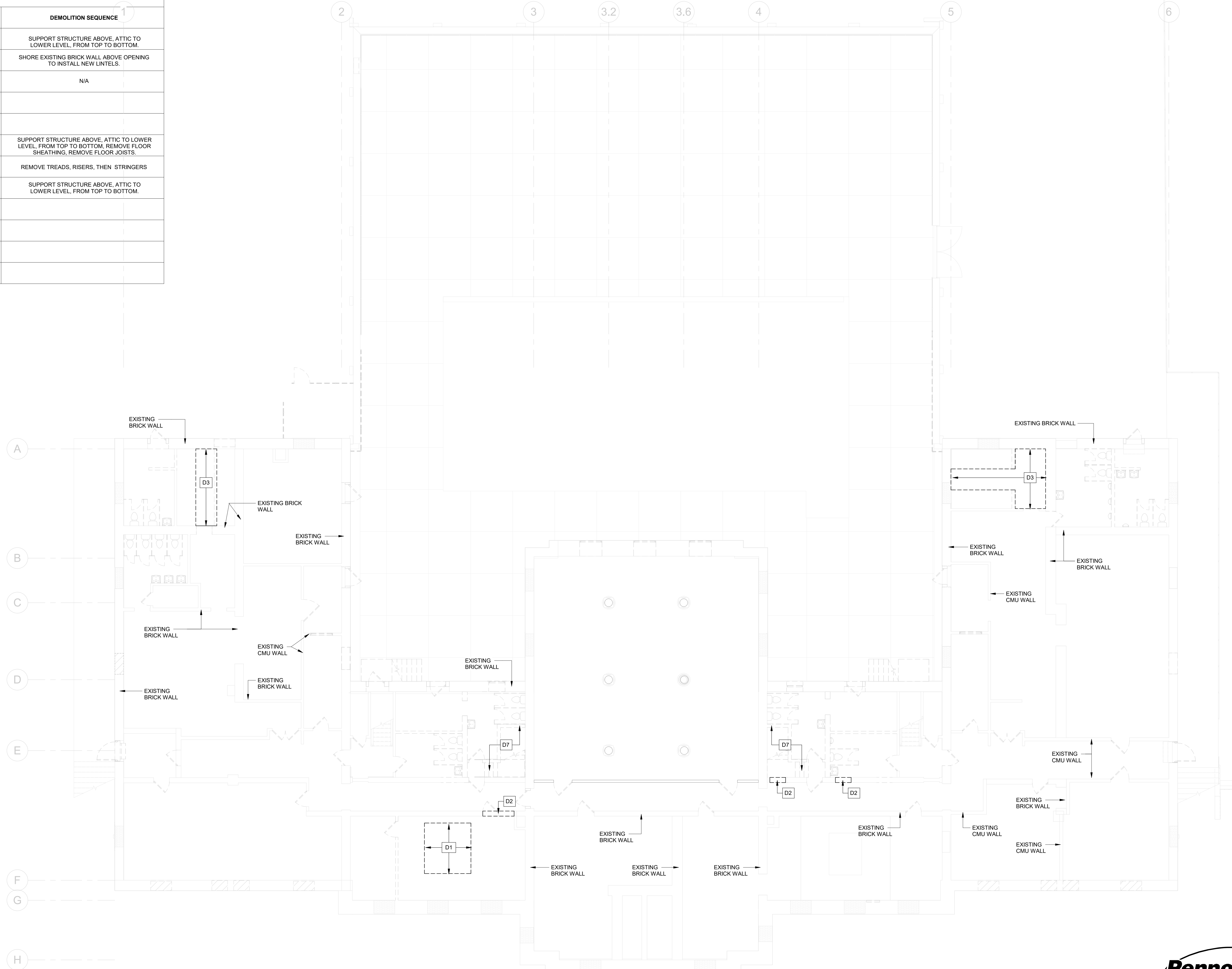


# DEMOLITION NOTES

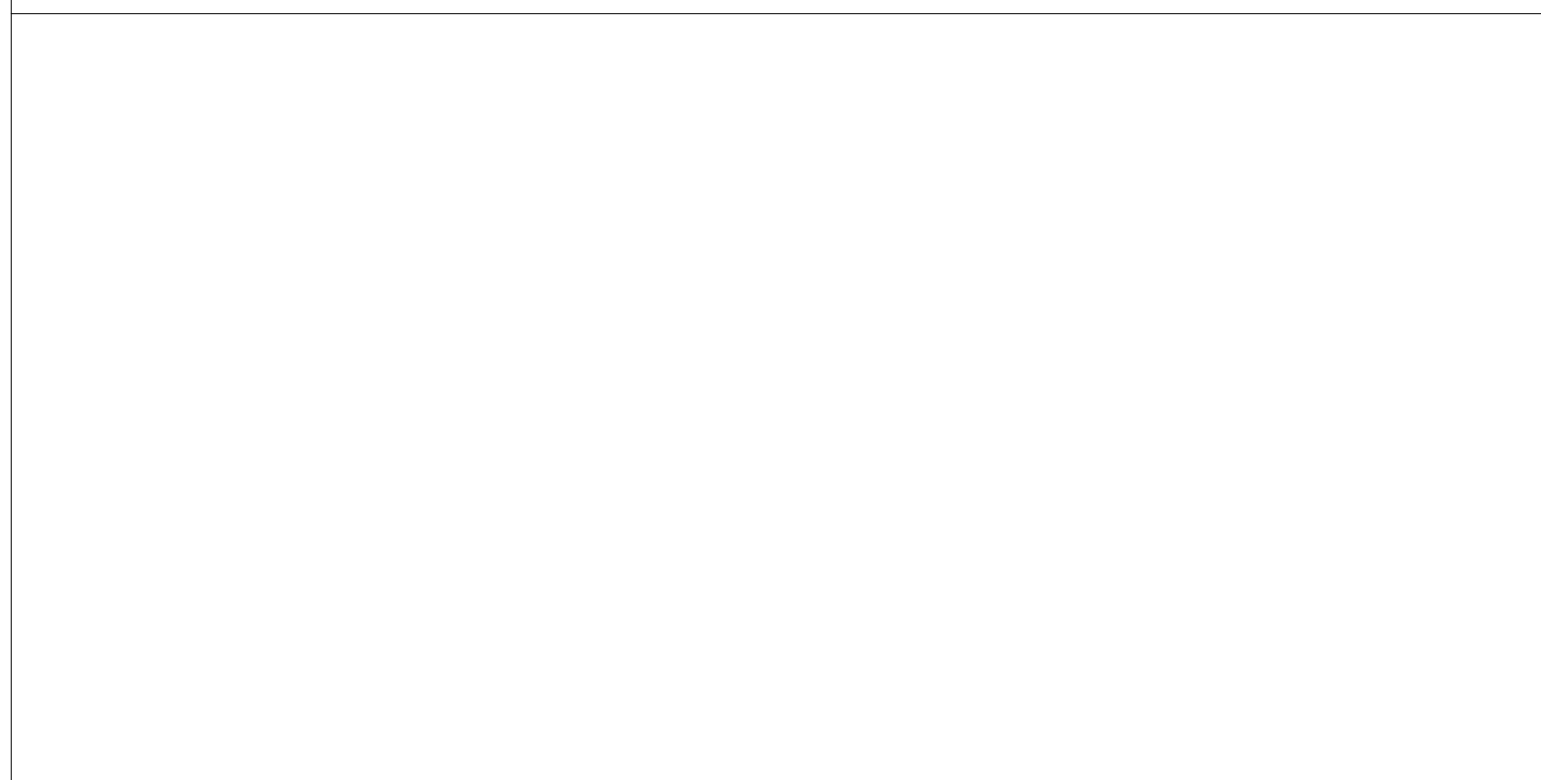
MARK	DEMOLITION NOTES	DEMOLITION SEQUENCE
D1	DEMO EXISTING SLAB ON GRADE FOR NEW ELEVATOR PIT.	SUPPORT STRUCTURE ABOVE, ATTIC TO LOWER LEVEL, FROM TOP TO BOTTOM.
D2	DEMO EXISTING LOAD BEARING WALL AND DOOR FOR NEW OPENING.	SHORE EXISTING BRICK WALL ABOVE OPENING TO INSTALL NEW LINTELS.
D3	REMOVE EXISTING RAISED CONCRETE FLOOR TO 1" BELOW EXISTING FLOOR SLAB. INTENTIONALLY ROUGHEN.	N/A
D4	NOT USED	
D5	NOT USED	
D6	DEMO EXISTING FLOOR FRAMING FOR NEW ELEVATOR. SHORE EXISTING REMAINING FRAMING, AS REQ'D.	SUPPORT STRUCTURE ABOVE, ATTIC TO LOWER LEVEL, FROM TOP TO BOTTOM, REMOVE FLOOR SHEATHING, REMOVE FLOOR JOISTS.
D7	DEMO EXISTING INTERIOR STAIR.	REMOVE TREADS, RISERS, THEN STRINGERS
D8	DEMO PORTION OF WOOD BEARING WALL ABOVE FLOOR SHOWN. SHORE EXISTING FLOOR FRAMING, AS REQ'D.	SUPPORT STRUCTURE ABOVE, ATTIC TO LOWER LEVEL, FROM TOP TO BOTTOM.
D9	NOT USED	
D10	NOT USED	
D11	NOT USED	
D12	NOT USED	



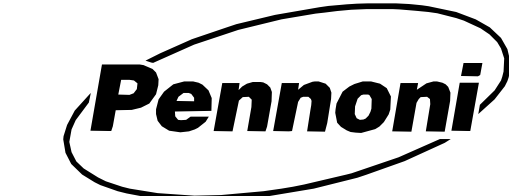
DEMO NOTE - D3



STAMP AREA



1 LOWER LEVEL DEMO PLAN  
1/8" = 1'-0"



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08/28/23

## REVISIONS

ISSUE	DATE	DESCRIPTION



REVIEWED BY:

PROJECT COORDINATOR

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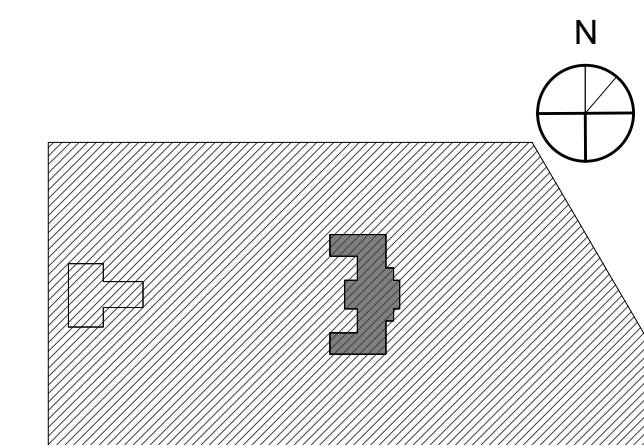
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PHILADELPHIA PENNSYLVANIA

PROJECT TITLE  
**KINGSESSING RECREATION CENTER BUILDING AND SITE IMPROVEMENTS - PACKAGE 2**

KEY PLAN



DRAWING TITLE

**DEMOLITION PLAN - LOWER LEVEL**

PROJECT NO. 21070

DATE 08/28/23

SCALE As indicated

DRAWN BY: W20

CHECKED BY: RM

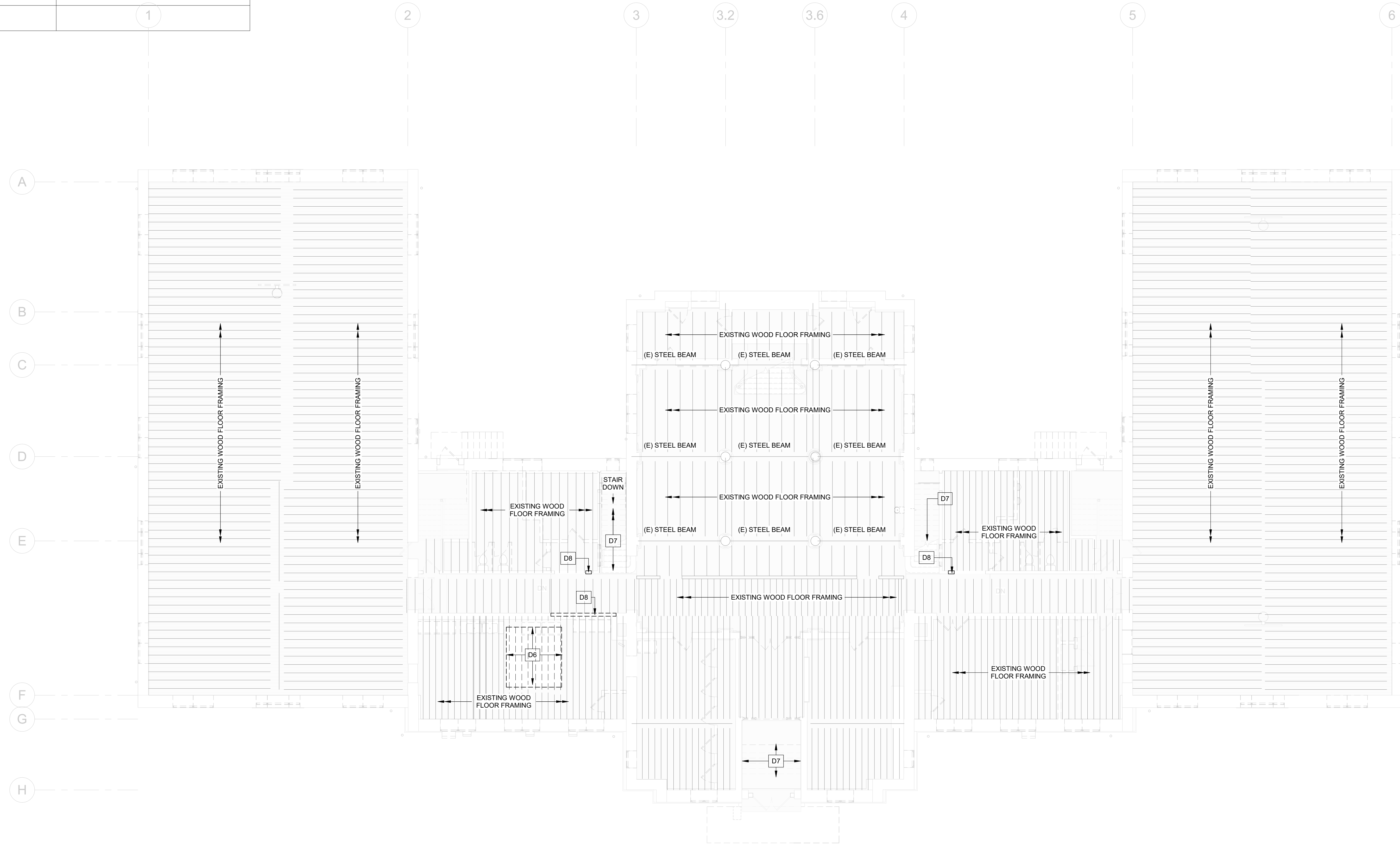
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DRAWING NO.  
**SD101-R.2**



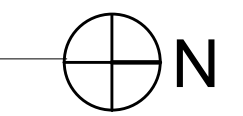
# DEMOLITION NOTES

MARK	DEMOLITION NOTES	DEMOLITION SEQUENCE
D1	DEMO EXISTING SLAB ON GRADE FOR NEW ELEVATOR PIT.	SUPPORT STRUCTURE ABOVE, ATTIC TO LOWER LEVEL, FROM TOP TO BOTTOM.
D2	DEMO EXISTING LOAD BEARING WALL AND DOOR FOR NEW OPENING.	SHORE EXISTING BRICK WALL ABOVE OPENING TO INSTALL NEW LINTELS.
D3	REMOVE EXISTING RAISED CONCRETE FLOOR TO 1" BELOW EXISTING FLOOR SLAB. INTENTIONALLY ROUGHEN.	N/A
D4	NOT USED	
D5	NOT USED	
D6	DEMO EXISTING FLOOR FRAMING FOR NEW ELEVATOR. SHORE EXISTING REMAINING FRAMING, AS REQ'D.	SUPPORT STRUCTURE ABOVE, ATTIC TO LOWER LEVEL, FROM TOP TO BOTTOM, REMOVE FLOOR SHEATHING, REMOVE FLOOR JOISTS.
D7	DEMO EXISTING INTERIOR STAIR.	REMOVE TREADS, RISERS, THEN STRINGERS
D8	DEMO PORTION OF WOOD BEARING WALL ABOVE FLOOR SHOWN. SHORE EXISTING FLOOR FRAMING, AS REQ'D.	SUPPORT STRUCTURE ABOVE, ATTIC TO LOWER LEVEL, FROM TOP TO BOTTOM.
D9	NOT USED	
D10	NOT USED	
D11	NOT USED	
D12	NOT USED	



STAMP AREA

1 FIRST FLOOR DEMO  
1/8" = 1'-0"



## REVISIONS

ISSUE	DATE	DESCRIPTION
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REVIEWED BY:  
PROJECT COORDINATOR:  
SEALS:



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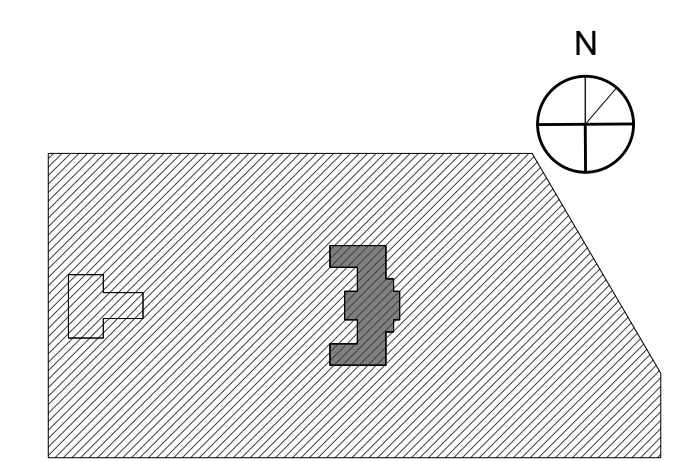
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PROJECT TITLE  
KINGSESSING RECREATION CENTER BUILDING AND SITE IMPROVEMENTS - PACKAGE 2

KEY PLAN



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PACKAGE 2 - IFB  
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08/28/23

DRAWING TITLE  
DEMOLITION PLAN - FIRST FLOOR

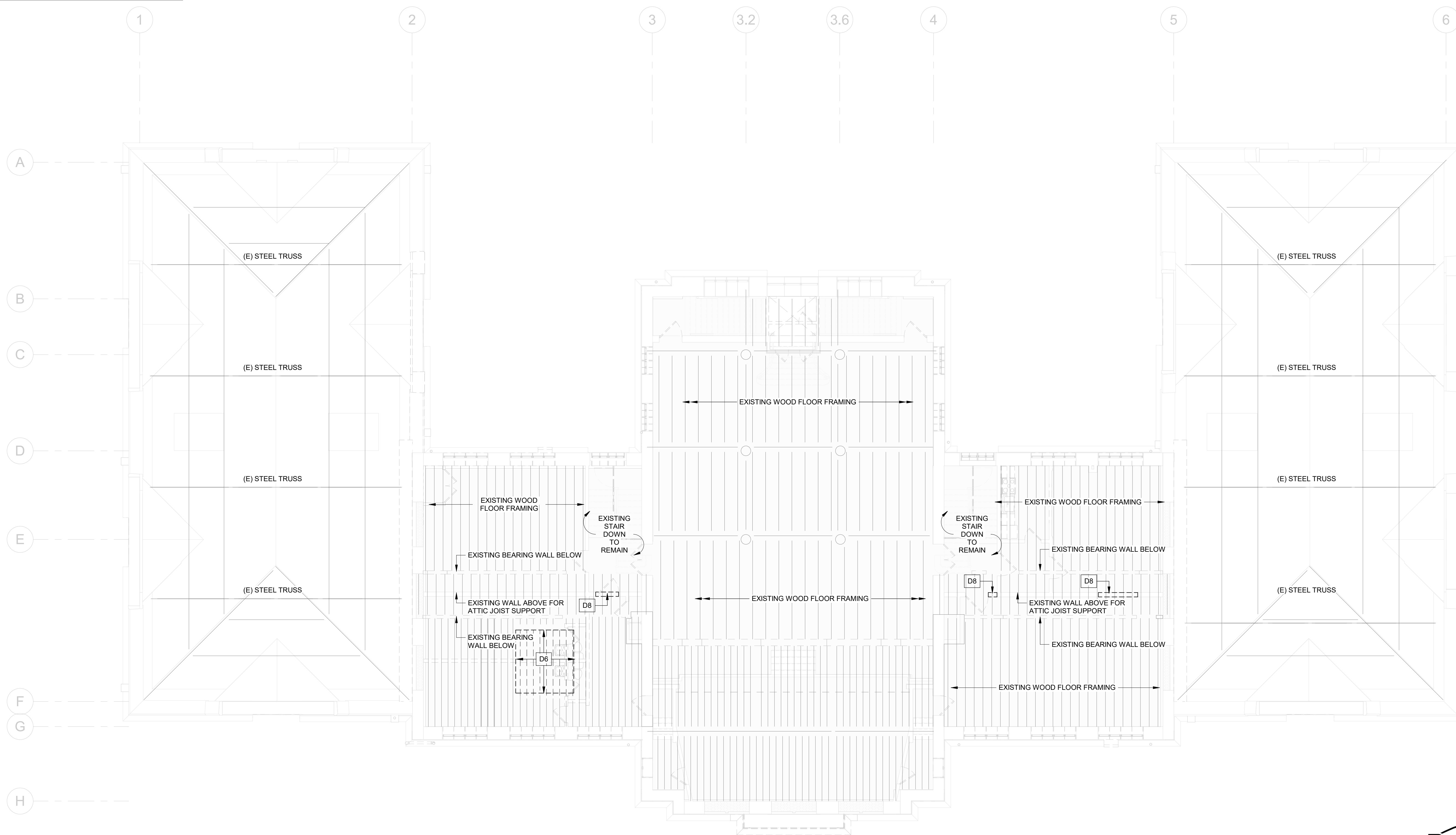
PROJECT NO.	DRAWING NO.
21070	SD-102-R.2
DATE: 08/28/23	SCALE: As indicated
DRAWN BY: WZD	CHECKED BY: PHU

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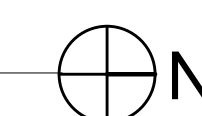
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D3	REMOVE EXISTING RAISED CONCRETE FLOOR TO 1" BELOW EXISTING FLOOR SLAB. INTENTIONALLY ROUGHEN.	N/A
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D5	NOT USED	
D6	DEMO EXISTING FLOOR FRAMING FOR NEW ELEVATOR. SHORE EXISTING REMAINING FRAMING, AS REQ'D.	SUPPORT STRUCTURE ABOVE, ATTIC TO LOWER LEVEL, FROM TOP TO BOTTOM, REMOVE FLOOR SHEATHING, REMOVE FLOOR JOISTS.
D7	DEMO EXISTING INTERIOR STAIR.	REMOVE TREADS, RISERS, THEN STRINGERS
D8	DEMO PORTION OF WOOD BEARING WALL ABOVE FLOOR SHOWN. SHORE EXISTING FLOOR FRAMING, AS REQ'D.	SUPPORT STRUCTURE ABOVE, ATTIC TO LOWER LEVEL, FROM TOP TO BOTTOM.
D9	NOT USED	
D10	NOT USED	
D11	NOT USED	
D12	NOT USED	



STAMP AREA

1 SECOND FLOOR DEMO PLAN  
1/8" = 1'-0"



## REVISIONS

ISSUE	DATE	DESCRIPTION
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PROJECT COORDINATOR

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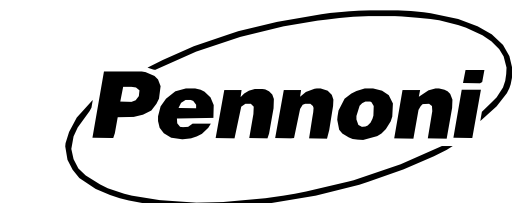
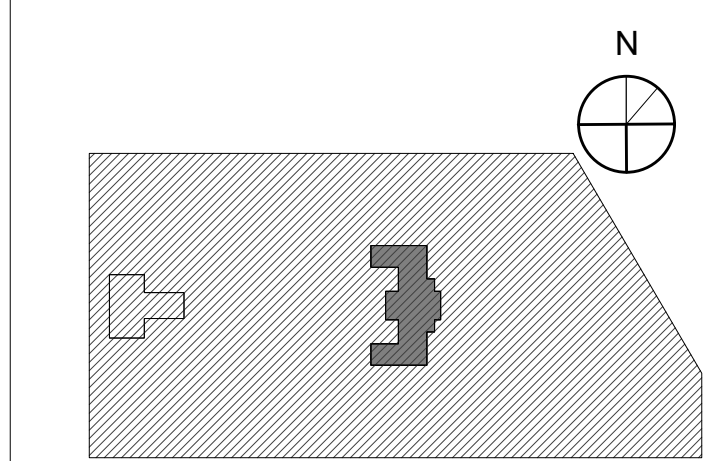
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CITY OF PHILADELPHIA  
REBUILD / PHILADELPHIA PARKS AND RECREATION  
1515 ARCH STREET  
11TH FLOOR, ONE PARKWAY BUILDING

PROJECT TITLE  
KINGSESSING RECREATION CENTER BUILDING AND SITE IMPROVEMENTS - PACKAGE 2

KEY PLAN



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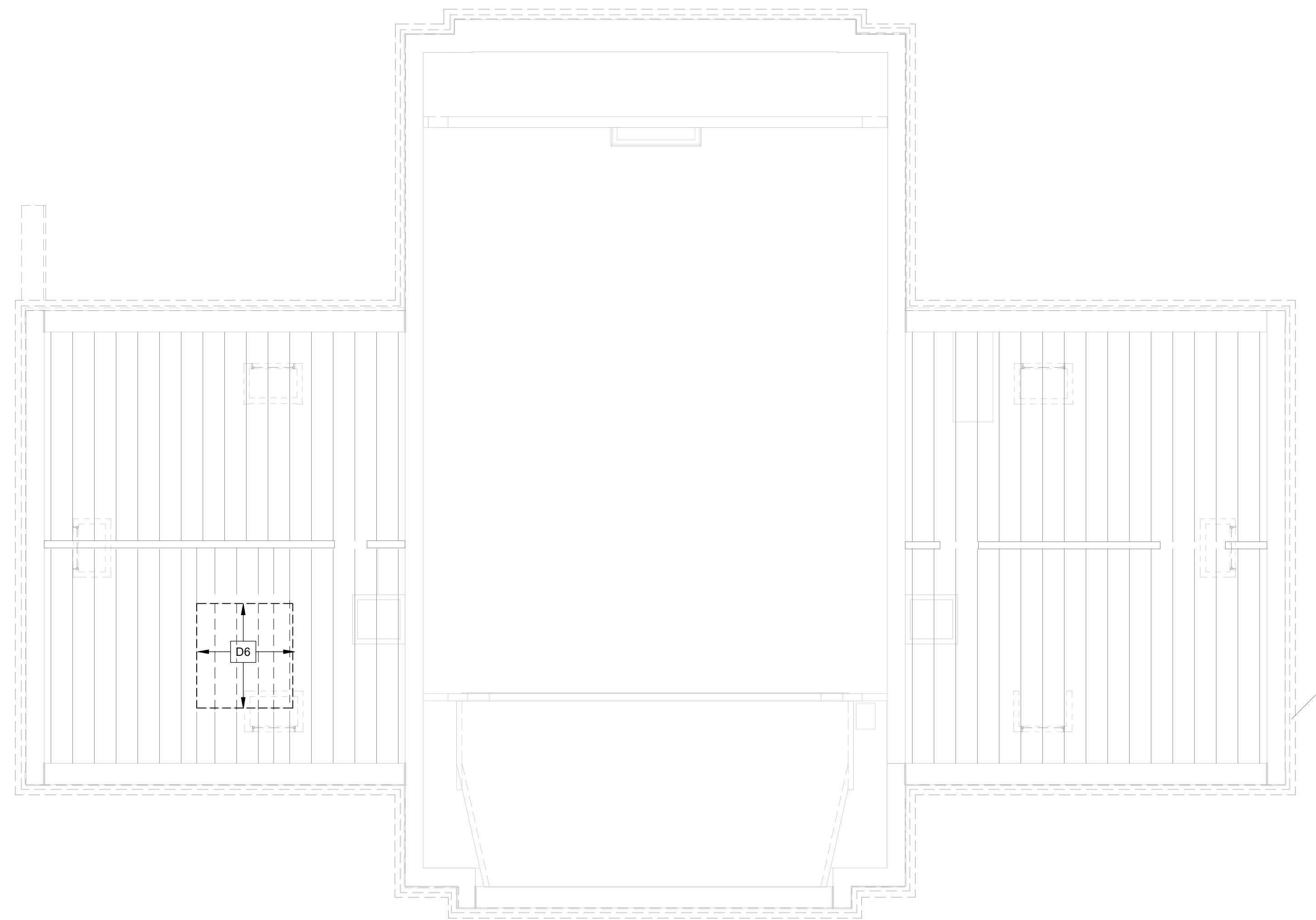
PACKAGE 2 - IFB  
NOT FOR CONSTRUCTION  
08/28/23

DRAWING TITLE	
DEMOLITION PLAN - SECOND FLOOR	
PROJECT NO.	DRAWING NO.
21070	
DATE	08/28/23
SCALE	As indicated
DRAWN BY:	W20
CHECKED BY:	PHU
NOTE: ALL DIMENSIONS AND CONDITIONS SHALL BE VERIFIED BY THE CONTRACTOR AT THE SITE BEFORE PROCEEDING WITH THE WORK.	

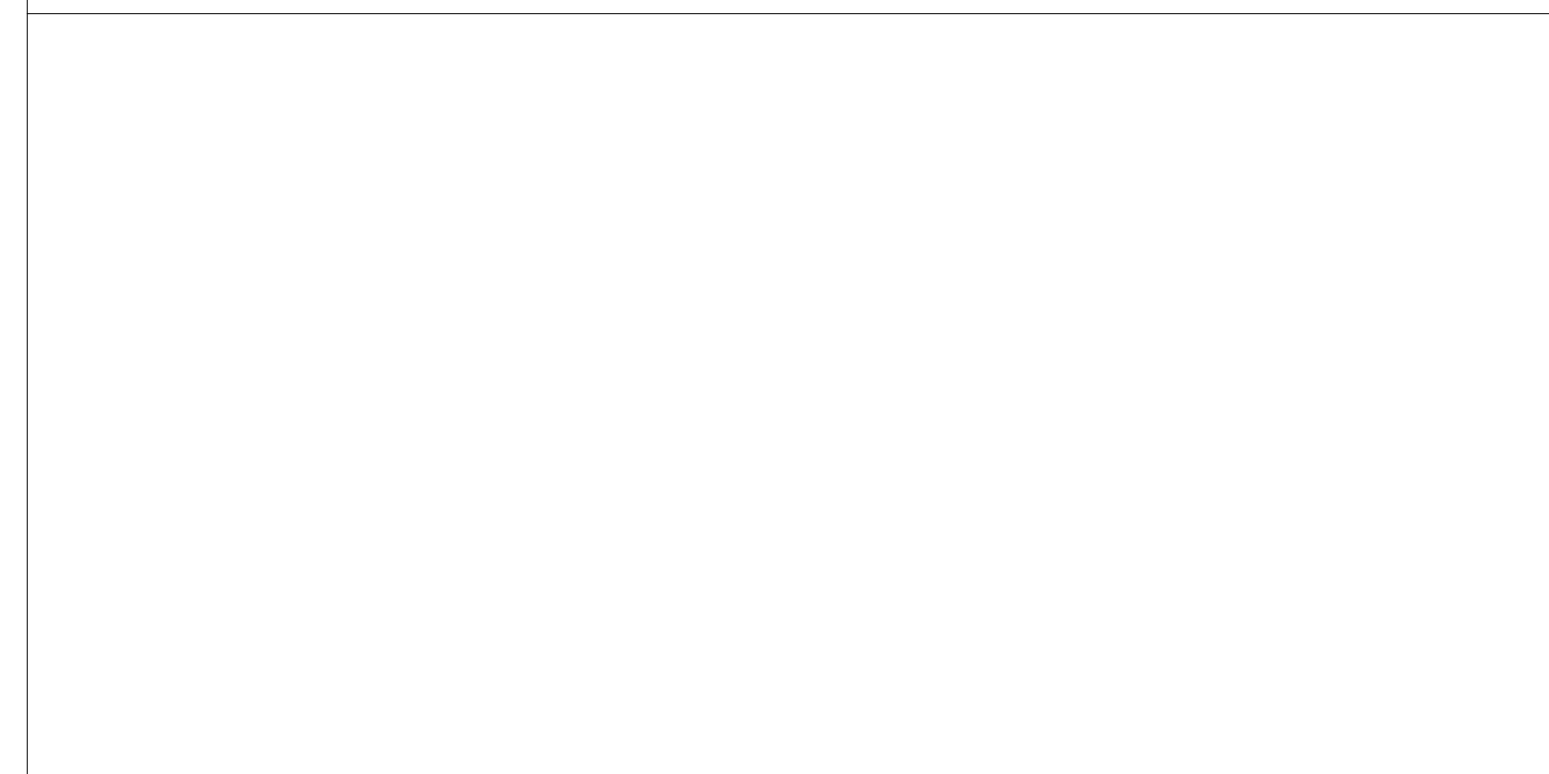
**SD-103-R.2**

# DEMOLITION NOTES

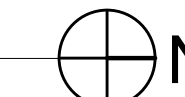
MARK	DEMOLITION NOTES	DEMOLITION SEQUENCE
D1	DEMO EXISTING SLAB ON GRADE FOR NEW ELEVATOR PIT.	SUPPORT STRUCTURE ABOVE, ATTIC TO LOWER LEVEL, FROM TOP TO BOTTOM.
D2	DEMO EXISTING LOAD BEARING WALL AND DOOR FOR NEW OPENING.	SHORE EXISTING BRICK WALL ABOVE OPENING TO INSTALL NEW LINTELS.
D3	REMOVE EXISTING RAISED CONCRETE FLOOR TO 1" BELOW EXISTING FLOOR SLAB. INTENTIONALLY ROUGHEN.	N/A
D4	NOT USED	
D5	NOT USED	
D6	DEMO EXISTING FLOOR FRAMING FOR NEW ELEVATOR. SHORE EXISTING REMAINING FRAMING, AS REQ'D.	SUPPORT STRUCTURE ABOVE, ATTIC TO LOWER LEVEL, FROM TOP TO BOTTOM. REMOVE FLOOR SHEATHING, REMOVE FLOOR JOISTS.
D7	DEMO EXISTING INTERIOR STAIR.	REMOVE TREADS, RISERS, THEN STRINGERS
D8	DEMO PORTION OF WOOD BEARING WALL ABOVE FLOOR SHOWN. SHORE EXISTING FLOOR FRAMING, AS REQ'D.	SUPPORT STRUCTURE ABOVE, ATTIC TO LOWER LEVEL, FROM TOP TO BOTTOM.
D9	NOT USED	
D10	NOT USED	
D11	NOT USED	
D12	NOT USED	



STAMP AREA



Ⓢ ATTIC DEMO PLAN  
1/8" = 1'-0"



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**NOT FOR CONSTRUCTION**  
**08/28/23**

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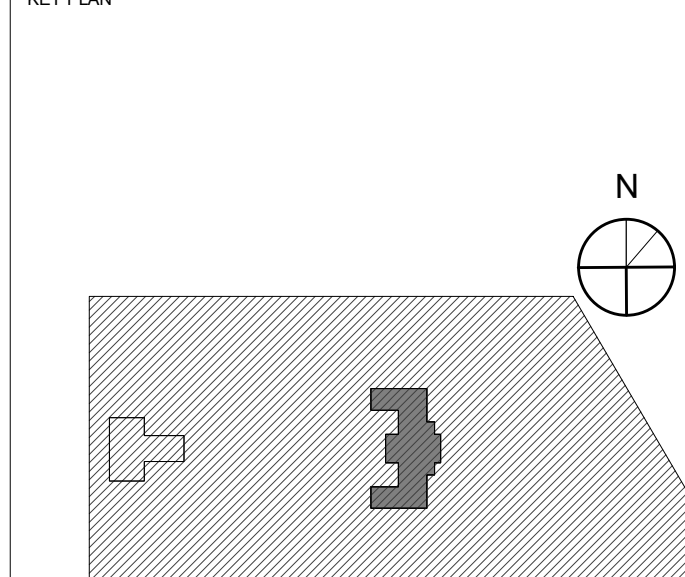
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PHILADELPHIA PENNSYLVANIA

PROJECT TITLE  
**KINGSESSING RECREATION CENTER BUILDING AND SITE IMPROVEMENTS - PACKAGE 2**

KEY PLAN



DRAWING TITLE  
**DEMOLITION PLAN - ATTIC**

PROJECT NO. <b>21070</b>	DRAWING NO.
-----------------------------	-------------

DATE 08/28/23	<b>SD-104-R.2</b>
SCALE As indicated	

DRAWN BY: WZD	
CHECKED BY: PM	

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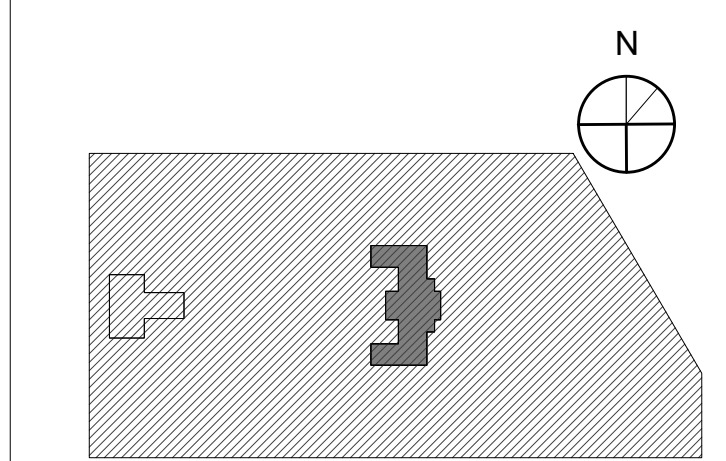
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**PROJECT TITLE**  
KINGSESSING RECREATION CENTER BUILDING AND SITE IMPROVEMENTS - PACKAGE 2

**KEY PLAN**



**DRAWING TITLE**  
LOWER LEVEL FRAMING PLAN

**PROJECT NO.** 21070 **DRAWING NO.**

**DATE** 09/28/23 **SCALE** As indicated

**DRAWN BY:** W20 **CHECKED BY:** P11

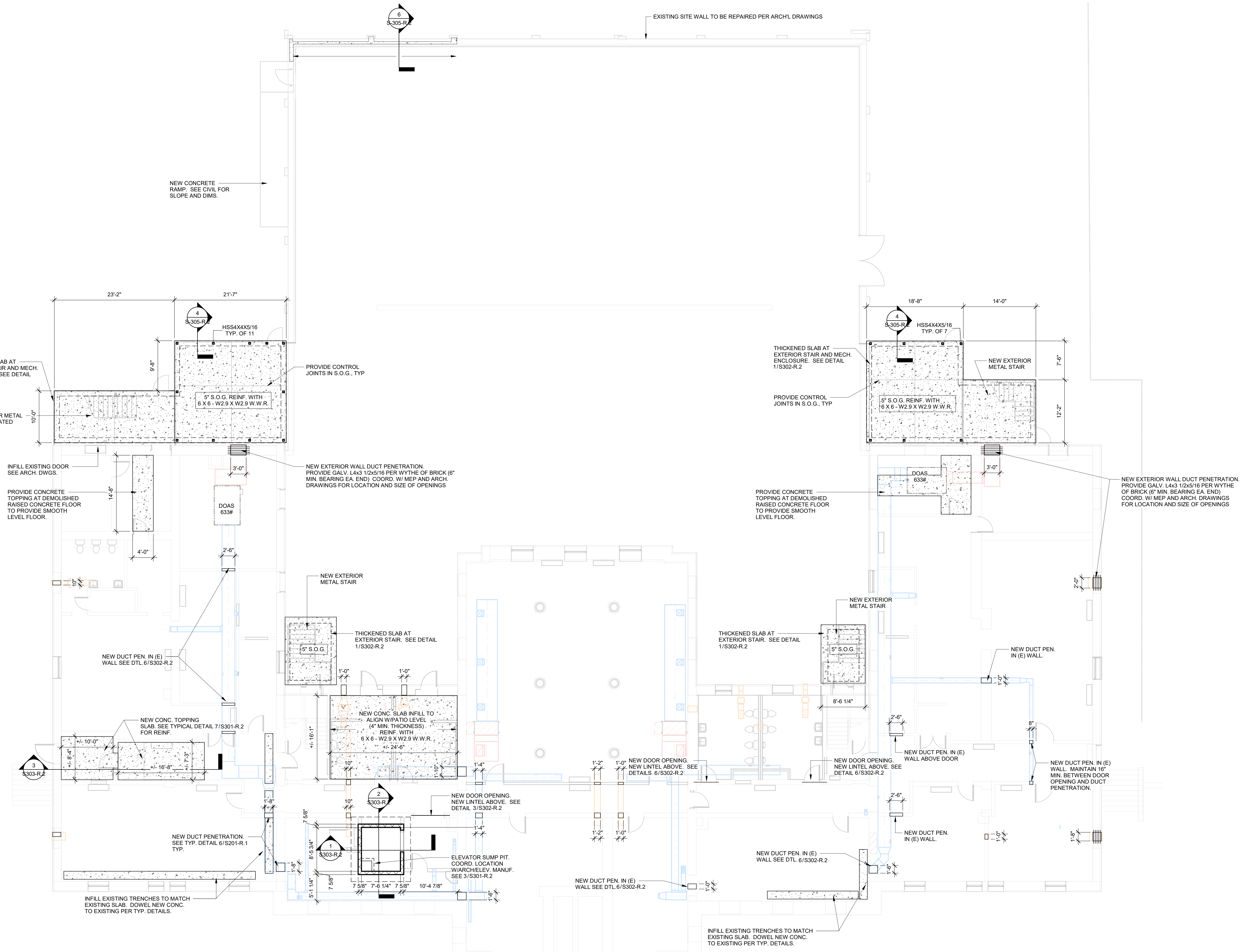
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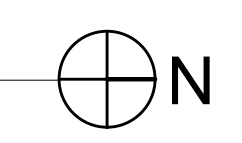
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**NOT FOR CONSTRUCTION**  
08/28/23



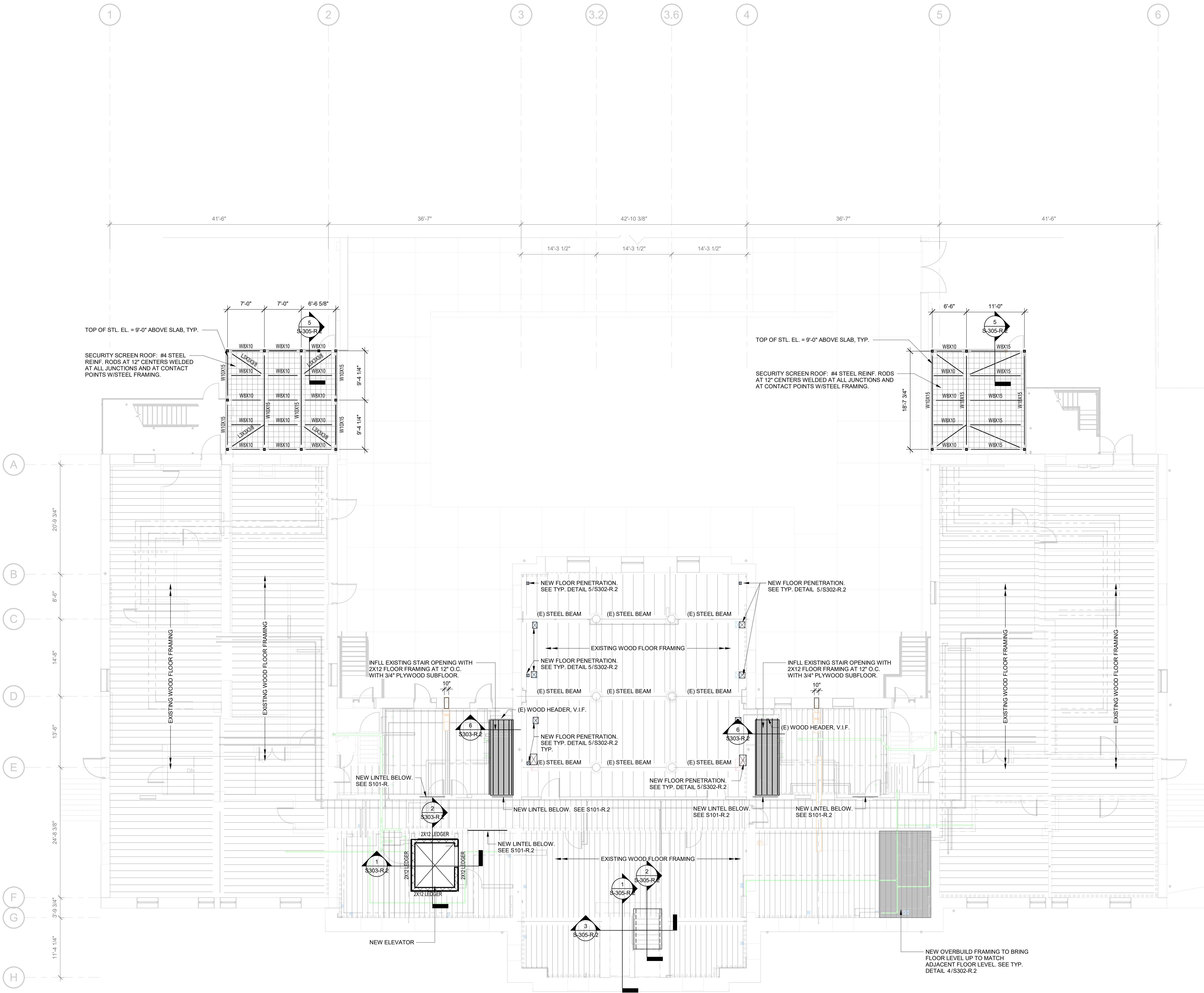
STAMP AREA

① LOWER LEVEL FOUNDATION PLAN  
1/8" = 1'-0"

- PLAN NOTES:**
- DATUM ELEVATION (0'-0") IS TOP OF EXISTING FIRST FLOOR SLAB.
  - ALL EXISTING DIMENSIONS SHALL BE VERIFIED IN THE FIELD.
  - GC TO COORDINATE FINAL DIMENSIONS WITH MANUFACTURER OR SUBCONTRACTOR.
  - SEE GENERAL STRUCTURAL NOTES FOR WORK INVOLVING MODIFICATION OF THE EXISTING STRUCTURE.
  - GRAY SHADED AREAS INDICATES WORK NOT IN CONTRACT.





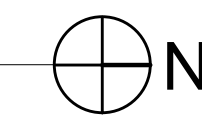


STAMP AREA

1 FIRST FLOOR  
1/8" = 1'-0"

**PLAN NOTES:**

1. DATUM ELEVATION (0'-0") IS TOP OF EXISTING FIRST FLOOR SLAB.
2. ALL EXISTING DIMENSIONS SHALL BE VERIFIED IN THE FIELD.
3. GC TO COORDINATE FINAL DIMENSIONS WITH MANUFACTURER OR SUBCONTRACTOR.
4. SEE GENERAL STRUCTURAL NOTES FOR WORK INVOLVING MODIFICATION OF THE EXISTING STRUCTURE.
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08/28/23

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ISSUE	DATE	DESCRIPTION



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PROJECT COORDINATOR:  
SEALS:



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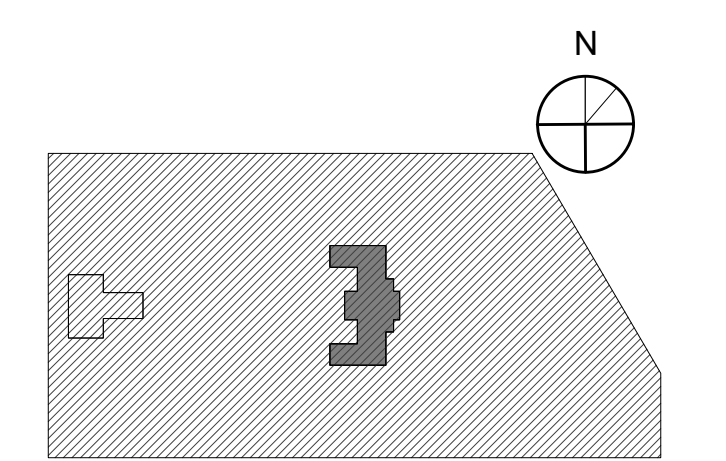
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11TH FLOOR, ONE PARKWAY BUILDING  
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PROJECT TITLE  
**KINGSESSING RECREATION CENTER BUILDING AND SITE IMPROVEMENTS - PACKAGE 2**

KEY PLAN



DRAWING TITLE  
**FIRST FLOOR FRAMING PLAN**

PROJECT NO. 21070 DRAWING NO. S102-R.2

DATE: 08/28/23  
SCALE: As indicated  
DRAWN BY: W20  
CHECKED BY: P11  
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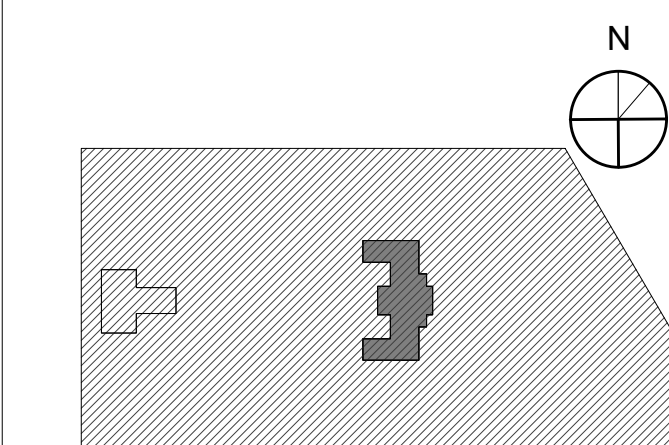
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KINGSESSING RECREATION CENTER BUILDING AND SITE IMPROVEMENTS - PACKAGE 2

KEY PLAN

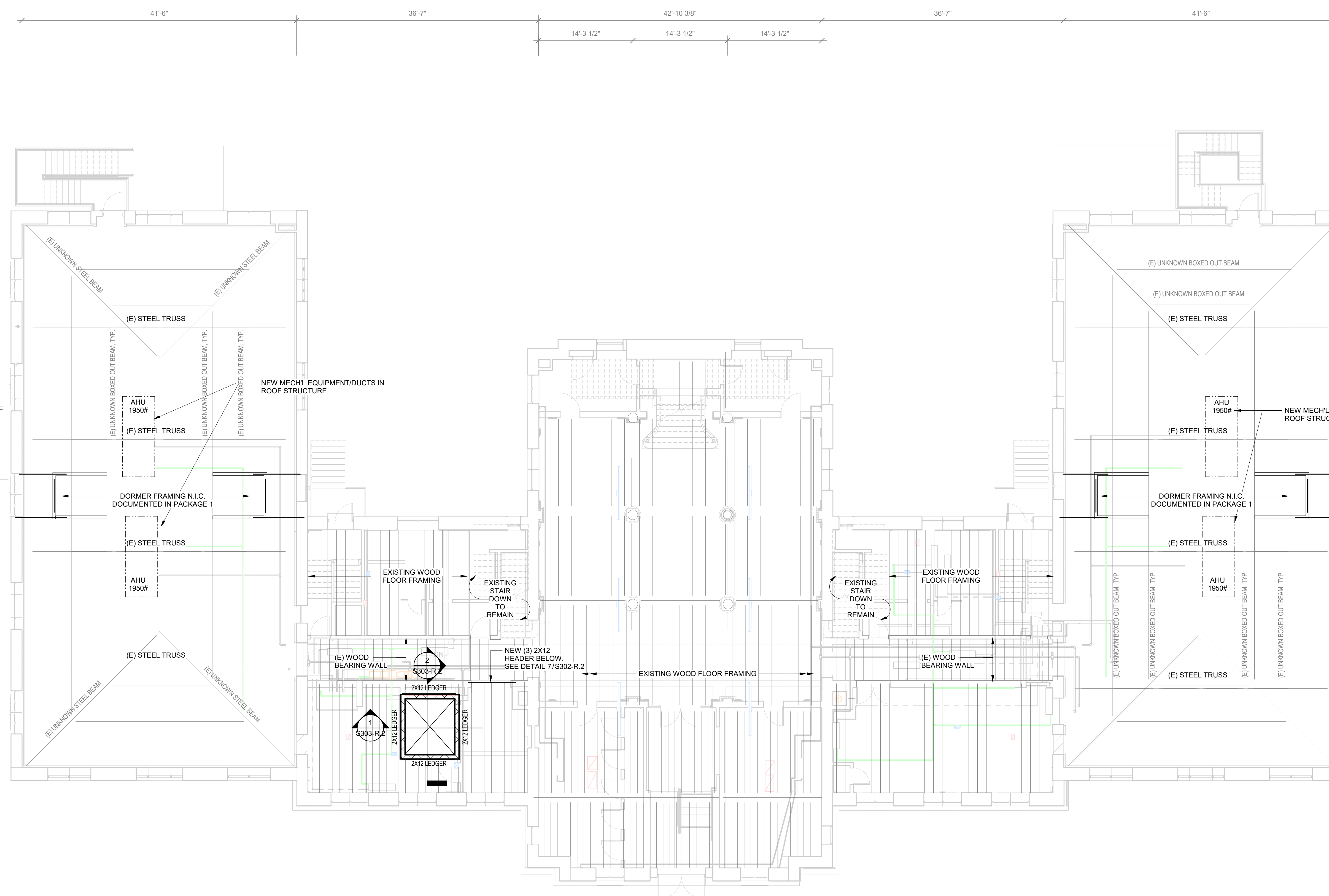
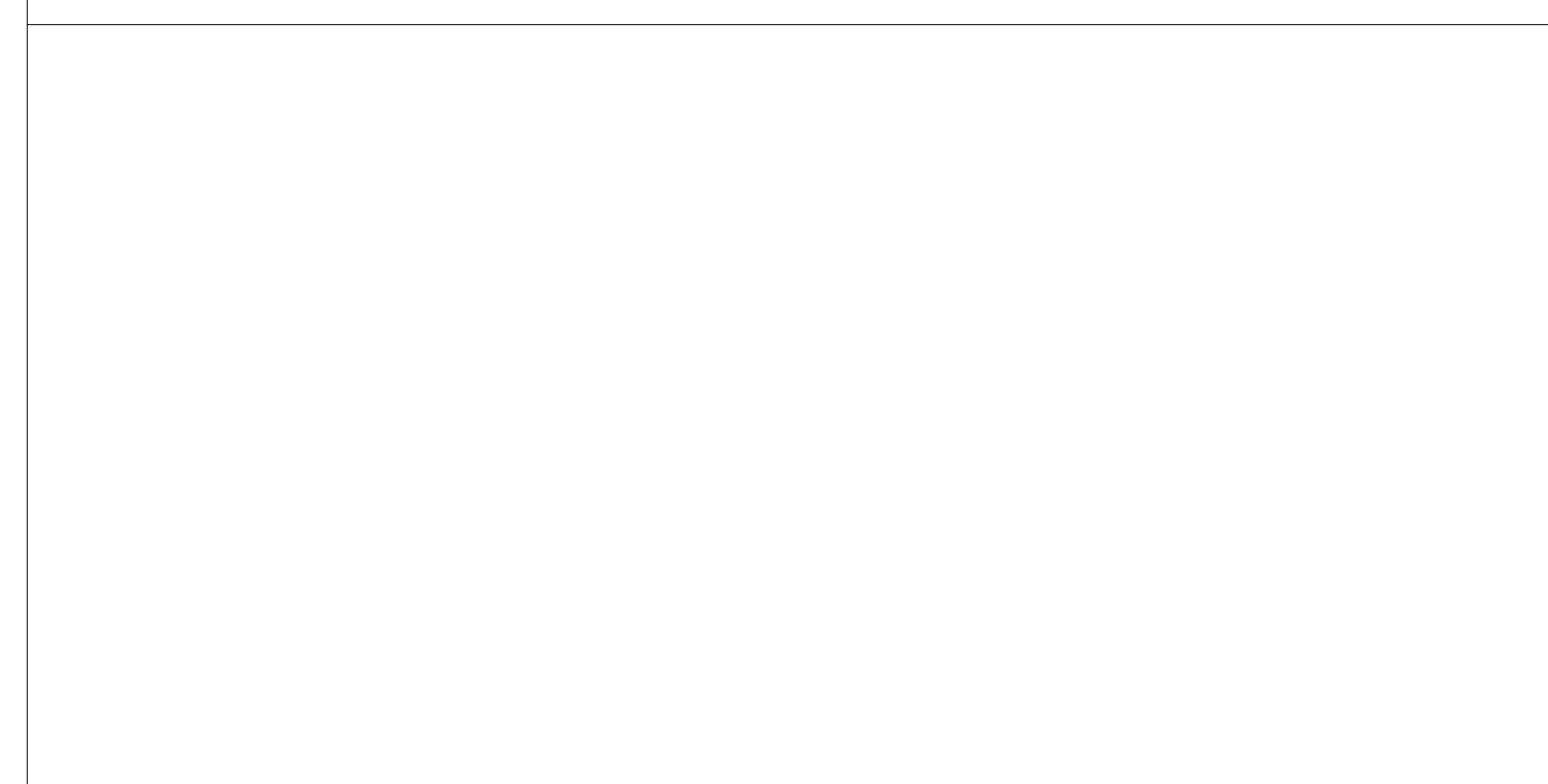


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08/28/23

STAMP AREA



SECOND FLOOR FRAMING PLAN  
1/8" = 1'-0"

PLAN NOTES:

- DATUM ELEVATION (0'-0") IS TOP OF EXISTING FIRST FLOOR SLAB.
- ALL EXISTING DIMENSIONS SHALL BE VERIFIED IN THE FIELD.
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- GRAY SHADED AREAS INDICATES WORK NOT IN CONTRACT.

DRAWING TITLE  
**SECOND FLOOR FRAMING PLAN**

PROJECT NO. <b>21070</b>	DRAWING NO. <b>S103-R.2</b>
DATE 08/28/23	
SCALE As indicated	
DRAWN BY W20	
CHECKED BY PBU	

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REVISIONS

ISSUE	DATE	DESCRIPTION



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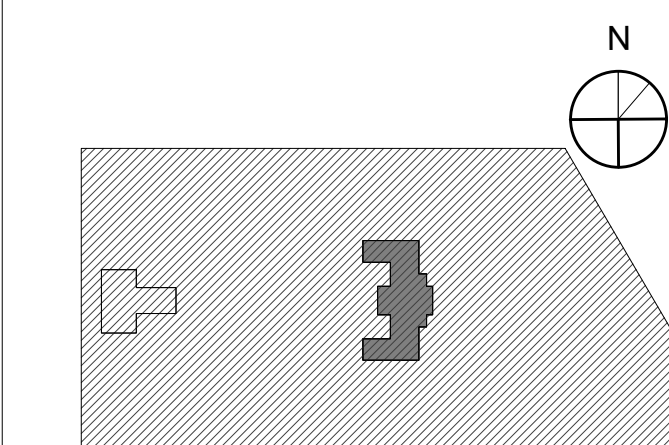
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PROJECT TITLE  
**KINGSESSING RECREATION CENTER BUILDING AND SITE IMPROVEMENTS - PACKAGE 2**

KEY PLAN



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PROJECT NO.

21070

DRAWING NO.

**S104-R.2**

DATE

09/28/23

SCALE

As indicated

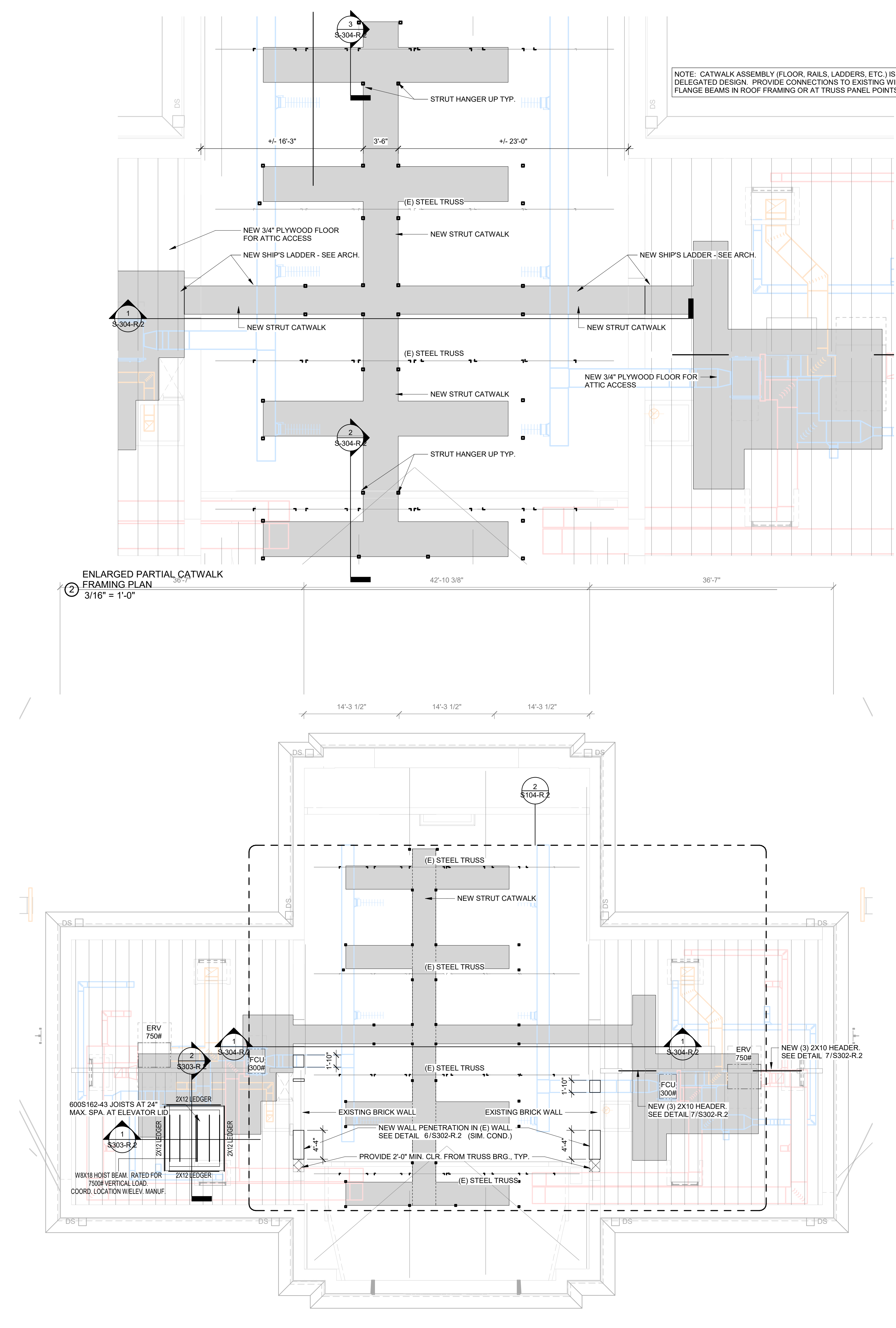
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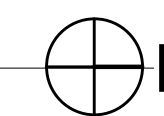


1 ATTIC FRAMING PLAN  
1/8" = 1'-0"

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STAMP AREA



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08/28/23



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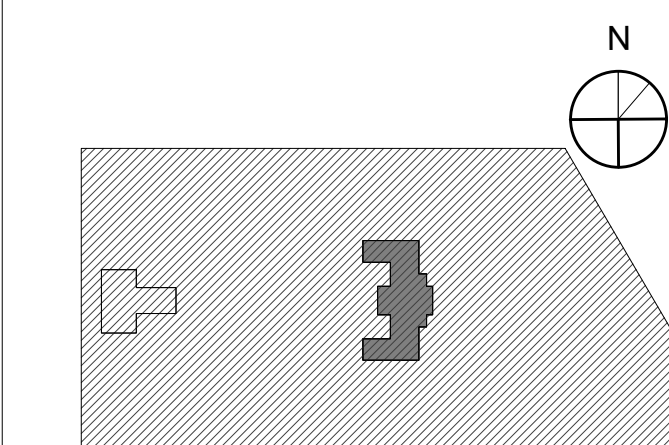
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PROJECT TITLE  
**KINGSESSING RECREATION CENTER BUILDING AND SITE IMPROVEMENTS - PACKAGE 2**

KEY PLAN



DRAWING TITLE  
**ROOF FRAMING PLAN**

PROJECT NO.  
**21070**

DATE  
08/28/23

SCALE  
As indicated

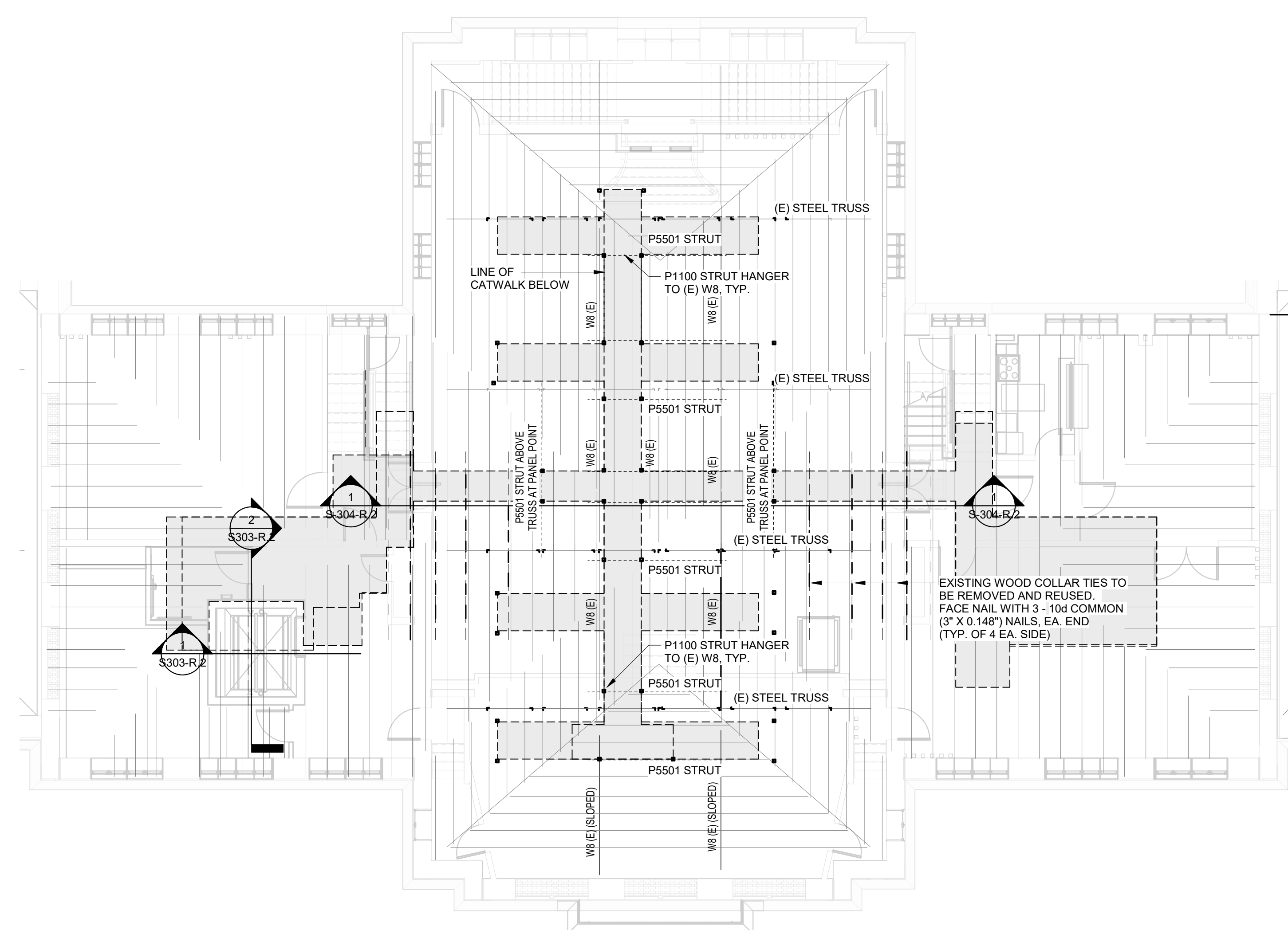
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CHECKED BY  
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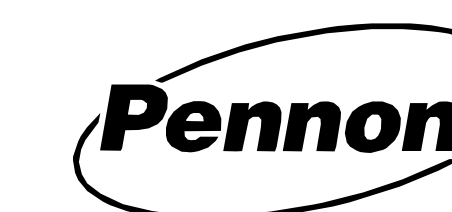
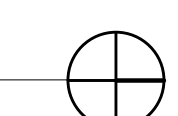
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**S105-R.2**



1 ROOF FRAMING PLAN  
1/8" = 1'-0"

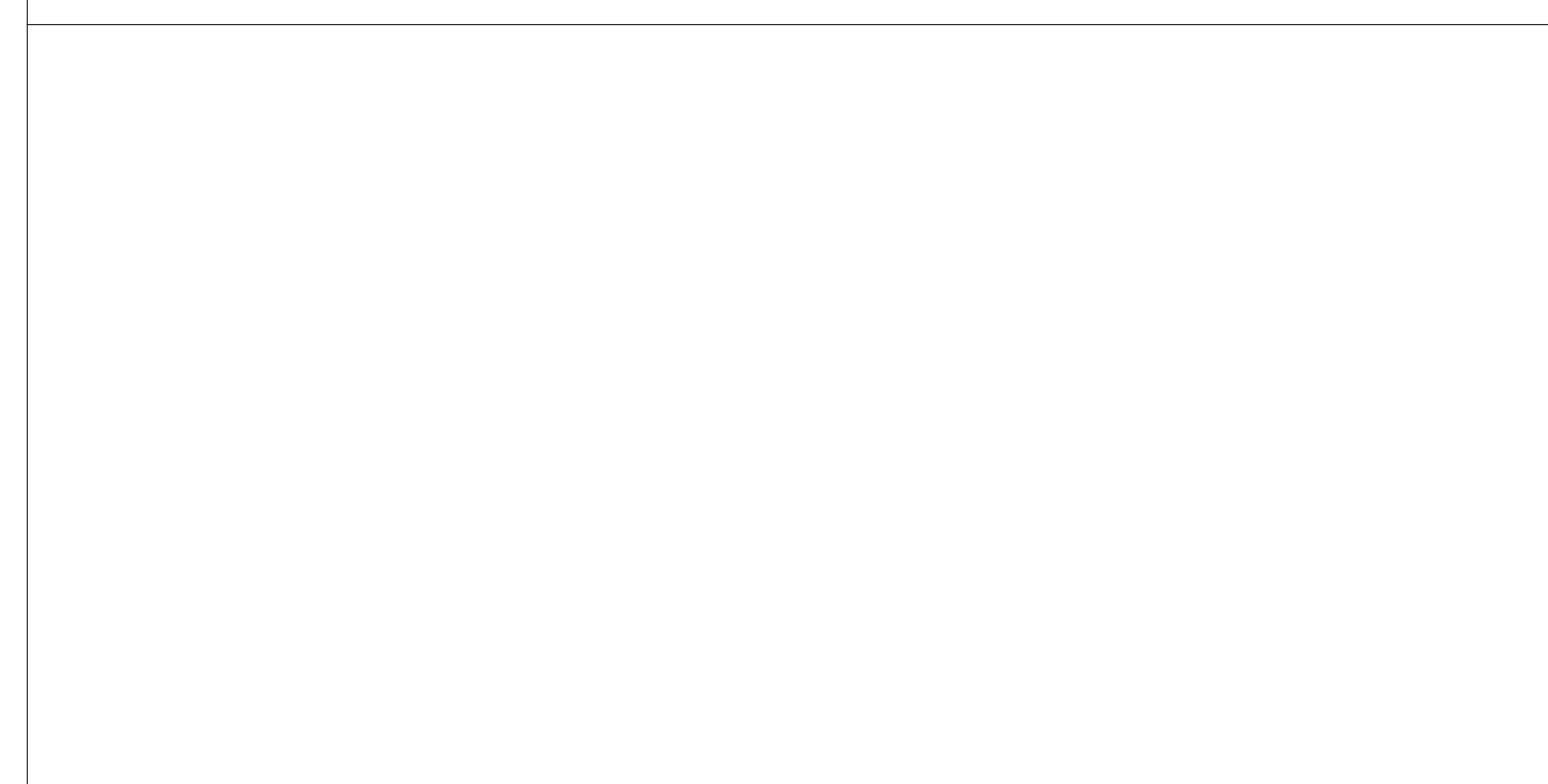


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STAMP AREA





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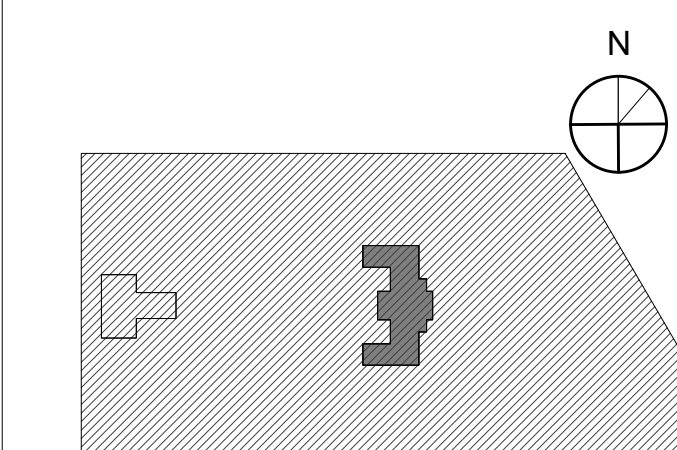
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REBUILD / PHILADELPHIA PARKS AND RECREATION  
1515 ARCH STREET  
11TH FLOOR, ONE PARKWAY BUILDING

PHILADELPHIA PENNSYLVANIA

PROJECT TITLE  
KINGSESSING RECREATION CENTER BUILDING AND SITE IMPROVEMENTS - PACKAGE 2

KEY PLAN



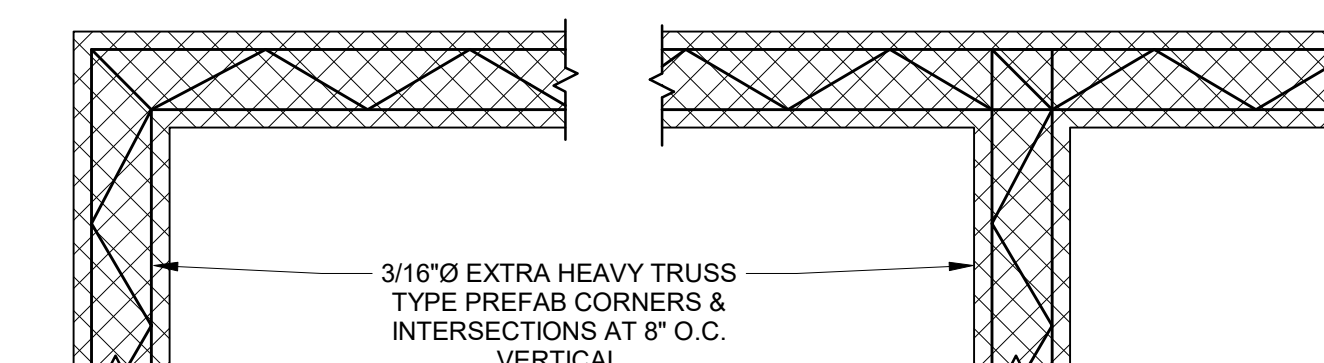
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08/28/23

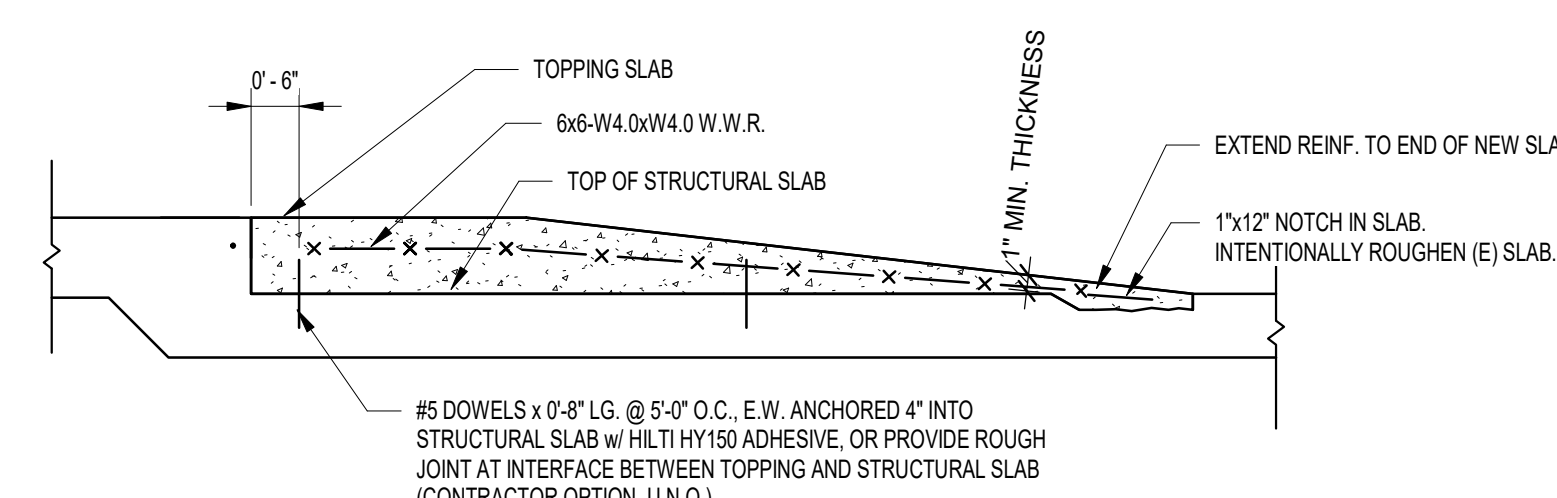
PROJECT NO.	DRAWING NO.
21070	
DATE	08/28/23
SCALE	As Indicated
DRAWN BY:	W20
CHECKED BY:	PHU
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**S301-R.2**



**6 MASONRY WALL HORIZONTAL JOINT REINFORCEMENT**  
3/4" = 1'-0"

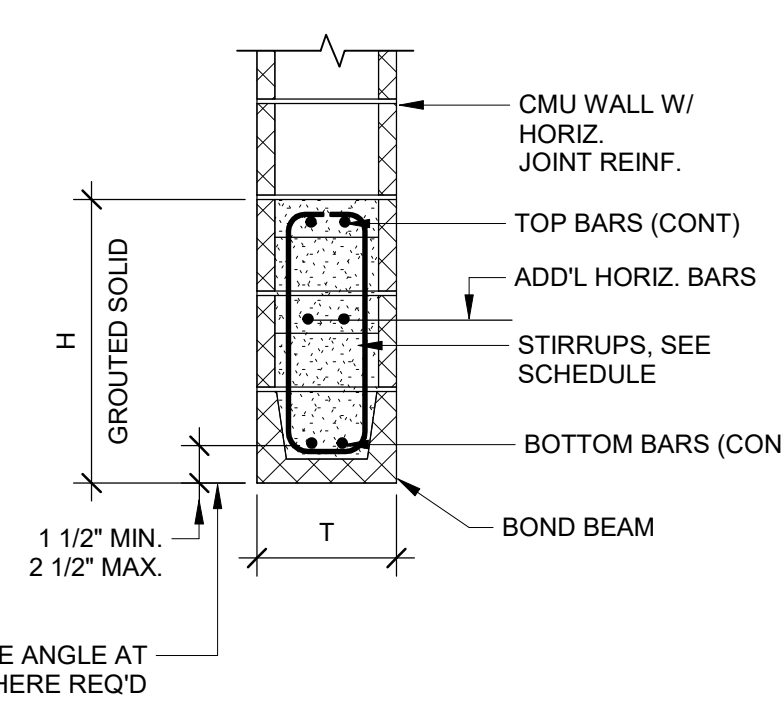
NOTES:  
A. CORNERS AND INTERSECTIONS UNLESS OTHERWISE NOTED OR SPECIFIED, AT POINTS WHERE CONCRETE MASONRY WALLS MEET OR INTERSECT, PLACE UNITS IN RUNNING BOND WITH ALTERNATE UNITS BEARING NOT LESS THAN 8 INCHES ON THE UNIT BELOW.



**7 TOPPING SLAB**  
1/2" = 1'-0"

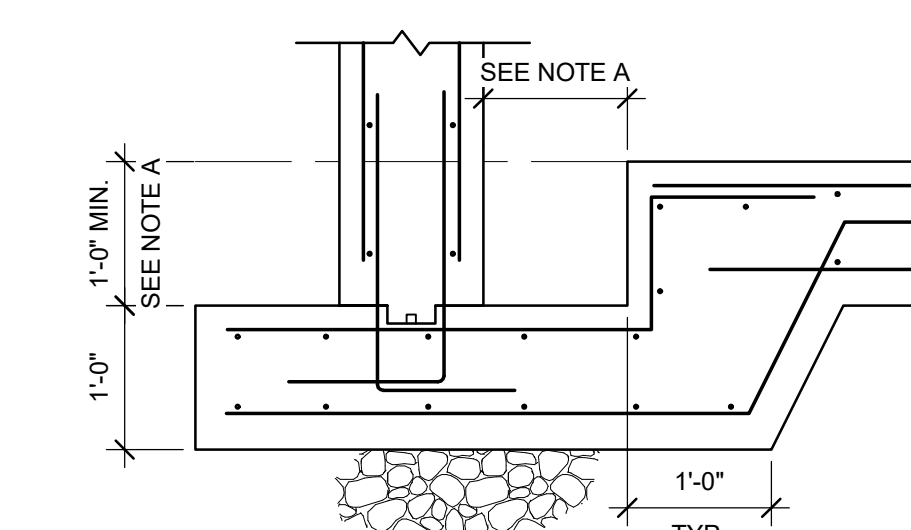
**LINTEL SCHEDULE:**

ML (72" MAX OPENING):  
16" DEEP, (1) #5 TOP AND BOT



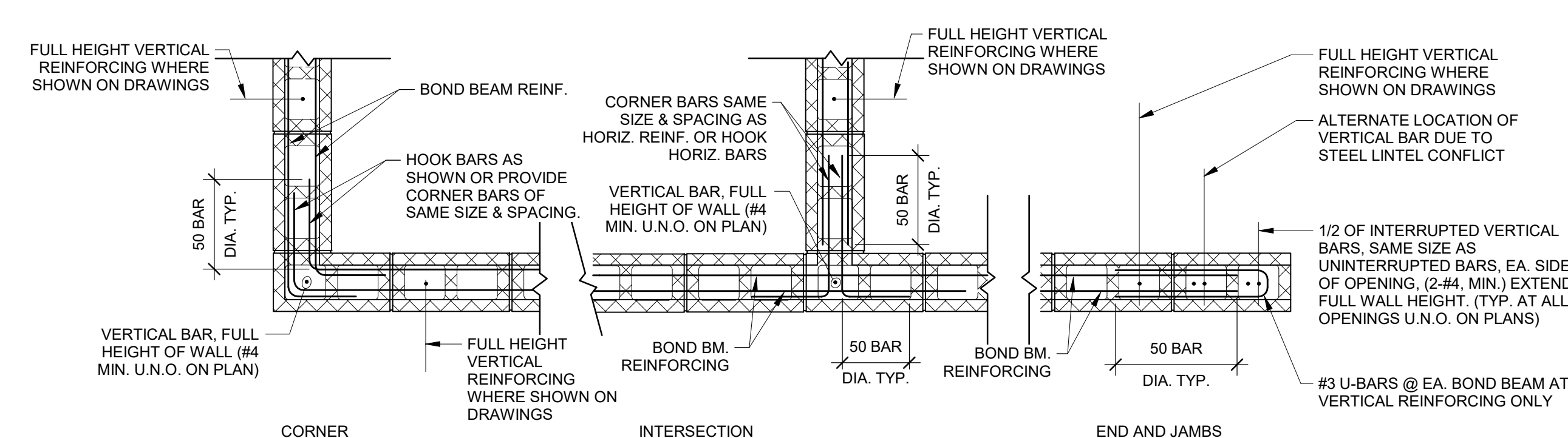
NOTES:  
1. REFER TO ARCHITECTURAL AND MEP DRAWINGS FOR LOCATIONS OF OPENINGS IN ALL CMU WALLS. FOR OPENINGS IN NON-STRUCTURAL CMU WALLS AND CMU VENEER, REFER TO THE LINTEL SCHEDULE NOTED IN GENERAL NOTES.  
2. PROVIDE VERTICAL WALL REINFORCING AT ALL JAMBS PER TYPICAL DETAILS. CONTINUE VERTICAL WALL REINFORCING THROUGH BOND BEAM LINTELS.  
3. PROVIDE 8" MINIMUM BEARING AT EACH END FOR BOND BEAM LINTELS.

**4 TYPICAL MASONRY WALL LINTEL**  
3/4" = 1'-0"



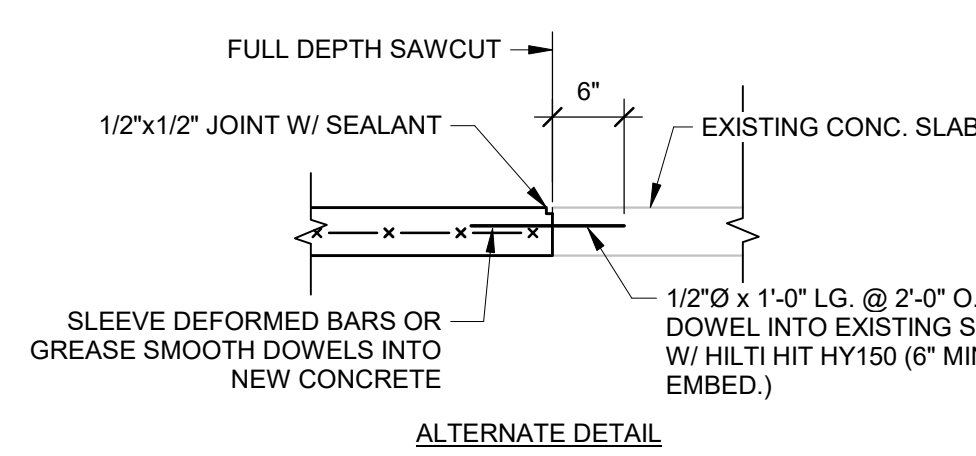
**3 ELEVATOR SUMP PIT**  
3/4" = 1'-0"

NOTES:  
A. COORD. W/ ELEVATOR MFR. FOR MINIMUM SUMP PIT DIMENSIONS.  
B. SEE ELEVATOR PIT DETAILS FOR ADD'L INFO NOT SHOWN.  
C. SEE ARCH. & M.E.P. DWGS. FOR LOCATION OF SUMP PIT.



**5 MASONRY WALL BOND BEAM REINFORCEMENT**  
3/4" = 1'-0"

NOTES:  
UNLESS OTHERWISE NOTED OR SPECIFIED, PROVIDE DOWELS FROM CONCRETE FOOTINGS OR WALLS BELOW, WITH SAME SIZE AND SPACING AS VERTICAL WALL REINFORCING. LAP DOWELS WITH VERTICAL REINFORCING 50 BAR DIAMETERS MINIMUM UNLESS NOTED OTHERWISE. GROUT SOLID CELLS WITH VERTICAL REINFORCING, FULL HEIGHT.



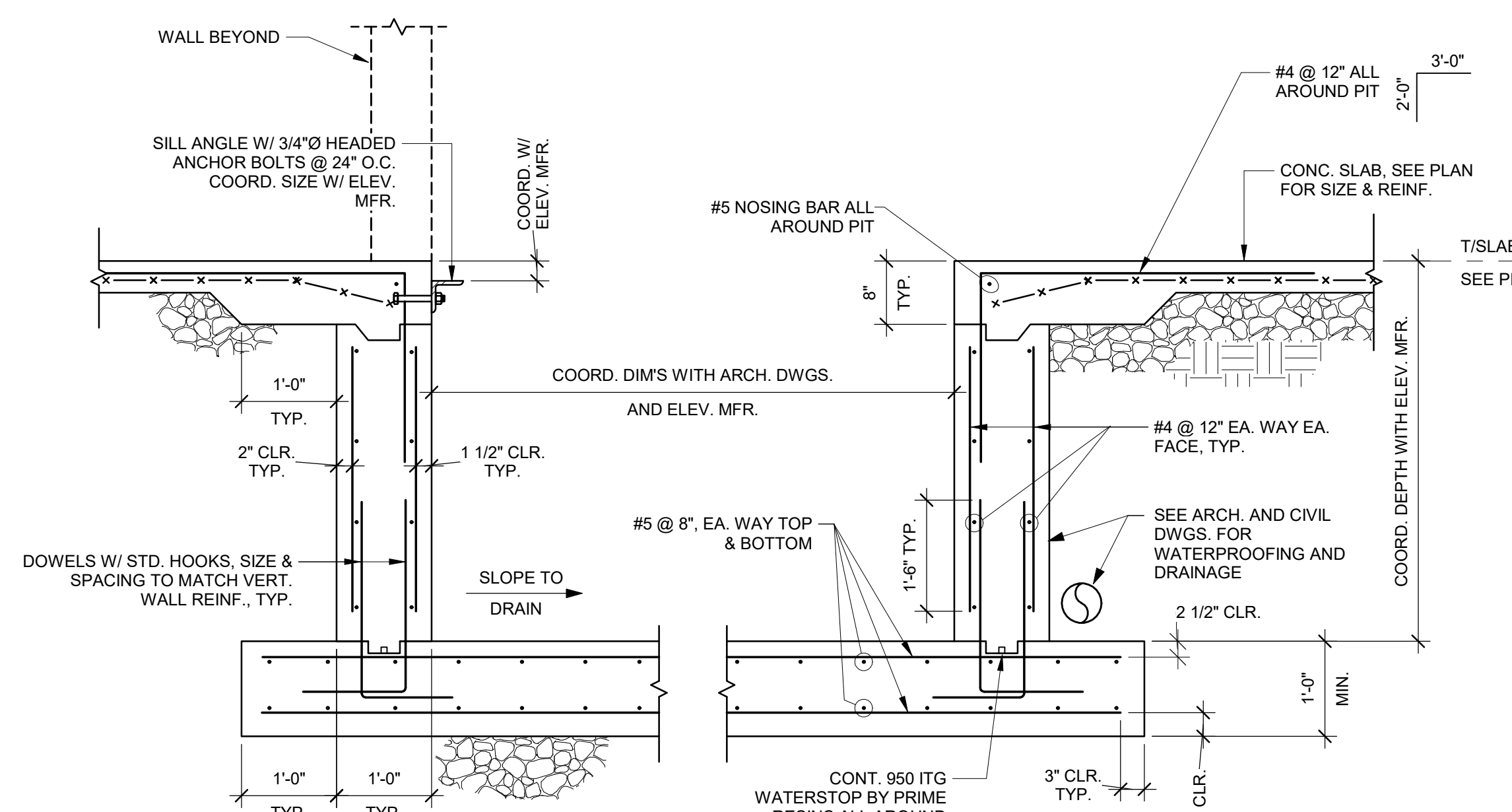
**2 NEW EXISTING SLAB JOINT**  
3/4" = 1'-0"

**EXISTING SLAB REMOVAL & REPLACEMENT NOTES**  
EXISTING CONCRETE SLABS ON GRADE SHALL BE SAWCUT AND REMOVED TO INSTALL NEW UTILITIES AND/OR FOUNDATIONS. REFER TO DRAWINGS OF THE VARIOUS TRADES FOR LOCATIONS. SLAB CUTTING SHALL RESULT IN A MAXIMUM SLOPE OF 1:1 FROM THE BOTTOM OF THE EXCAVATION TO THE EDGE OF THE REMAINING EXISTING SLAB. THE EXTENT OF SLAB REMOVAL MUST BE DETERMINED BY THE CONTRACTOR BASED UPON THE DEPTHS AND SIZES OF EXCAVATIONS. IN ANY LOCATION WHERE SLAB CUTTING RESULTS IN STRIPS OF EXISTING REMAINING SLABS LESS THAN 10 FEET WIDE, REMOVE THE SLAB BETWEEN THE ADJACENT CUTS. DO NOT CUT INTO OR DAMAGE ANY LOAD BEARING CONCRETE FOUNDATION.

AFTER INSTALLATION OF UTILITIES AND/OR FOUNDATIONS, BACKFILL AND COMPACT WITH APPROVED, ENGINEERED FILL OR WITH "CONTROLLED LOW STRENGTH MATERIAL (CLSM). CLSM SHALL HAVE A COMPRESSIVE STRENGTH OF A MINIMUM OF 50 - 120 PSI.

DRILL AND EPOXY GROUT #4 x 1'-0" LG. REBAR AT MID-HEIGHT OF EXISTING SLAB. SPACE AT 2'-0" O.C. MAXIMUM AND EMBED 6 INCHES INTO EXISTING ADJACENT SLABS.

INSTALL NEW CONCRETE SLAB ON GRADE (THICKNESS TO MATCH EXISTING OR AS NOTED ON PLANS, (4" MIN.)), REINFORCED WITH 6#6 W2.0 WWR OVER 6" CRUSHED STONE. PROVIDE JOINTS IN SLABS PER TYPICAL DETAILS.



**1 CONCRETE SINGLE ELEVATOR PIT DETAIL**  
3/4" = 1'-0"

NOTE:  
1. REFER TO CONCRETE WALL CORNER DETAILS FOR ADD'L INFO.

STAMP AREA



REVISIONS

ISSUE	DATE	DESCRIPTION



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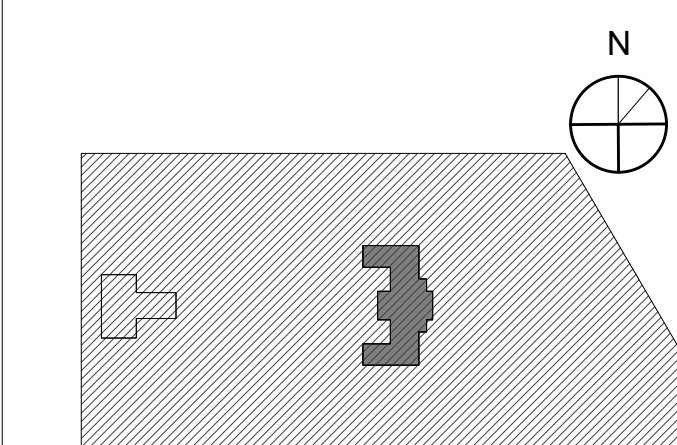
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KINGSESSING RECREATION CENTER BUILDING AND SITE IMPROVEMENTS - PACKAGE 2

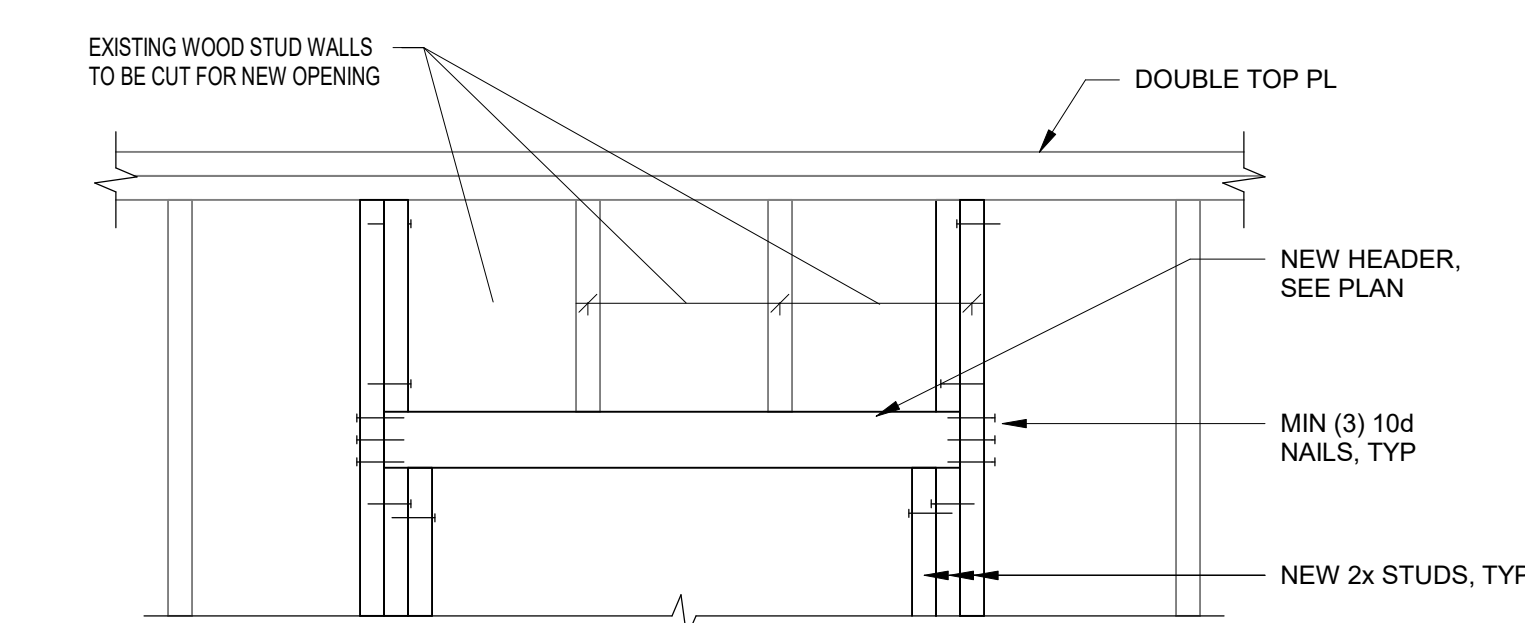
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7 TYPICAL INTERIOR HEADER DETAIL IN EXISTING WALL  
1" = 1'-0"

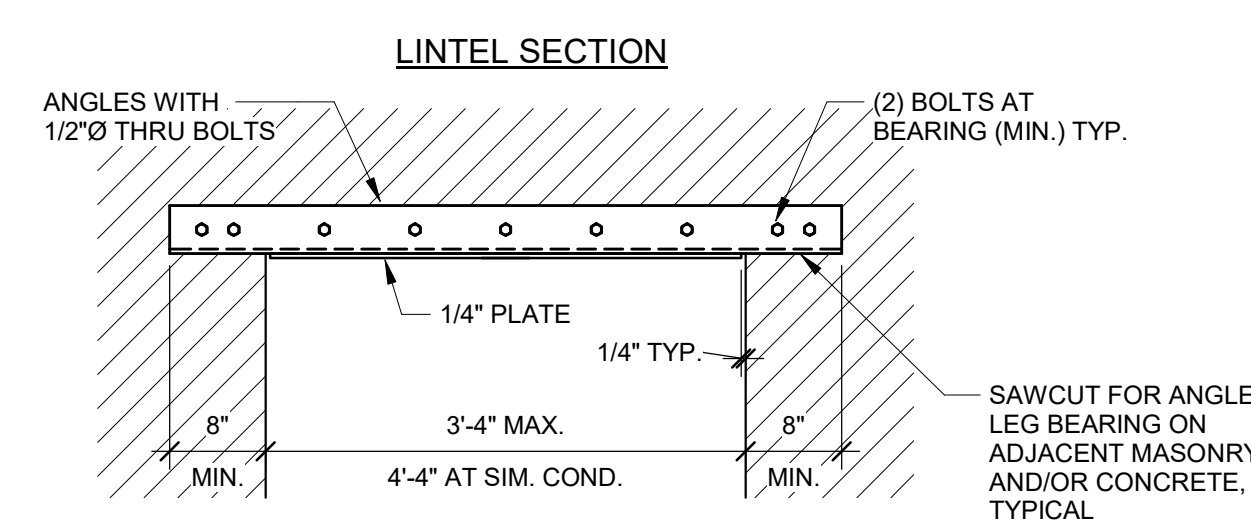
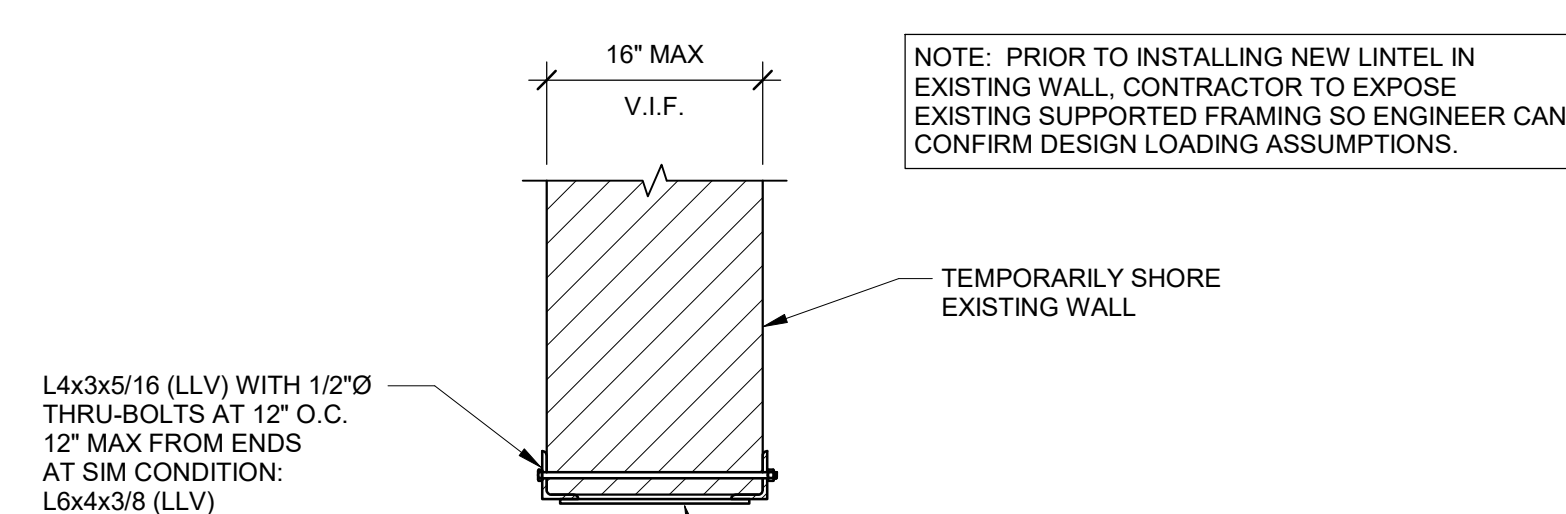
MISCELLANEOUS BOND OR PRECAST MASONRY LINTEL SCHEDULE			
WALL THICKNESS	MASONRY OPENING UP TO 6'-0"	MASONRY OPENING 6'-1" TO 8'-0"	MASONRY OPENING 8'-1" TO 10'-0"
6" WALL	6"x8" CONCRETE WITH (1) - #5 TOP AND BOTTOM		
8" WALL	8"x8" CONCRETE WITH (2) - #4 TOP AND BOTTOM OR 8"x8" BOND BEAM WITH (1) - #5 TOP AND BOTTOM	8"x8" CONCRETE WITH (2) - #4 TOP AND BOTTOM OR 8"x16" BOND BEAM WITH (1) - #6 TOP AND BOTTOM	8"x12" CONCRETE WITH (2) - #5 TOP AND BOTTOM OR 8"x16" BOND BEAM WITH (2) - #5 TOP AND BOTTOM

NOTE:  
1. PROVIDE MINIMUM 8" BEARING ON BRICK, SOLID OR GROUTED SOLID CONCRETE BLOCK.  
2. REFER TO ARCH AND MECH DRAWINGS FOR LOCATION AND SIZE OF FOR NON-BEARING MASONRY WALL.

MISCELLANEOUS STEEL ANGLE MASONRY WALL LINTEL SCHEDULE			
WALL THICKNESS	MASONRY OPENING UP TO 4'-0"	MASONRY OPENING 4'-1" TO 6'-0"	MASONRY OPENING 6'-1" TO 8'-0"
6" WALL	JL 3 1/2x3 1/2x5/16	JL 3 1/2x2 1/2x5/16	JL 3 1/2x2 1/2x3/8
8" WALL	JL 3 1/2x3 1/2x5/16	JL 4x3 1/2x5/16	JL 6x3 1/2x3/8
12" WALL OR GREATER	L 3 1/2x3 1/2x5/16 PER 4" WIDTH OF WALL	L 4x3 1/2x5/16 PER 4" WIDTH OF WALL	L 6x3 1/2x3/8 PER 4" WIDTH OF WALL

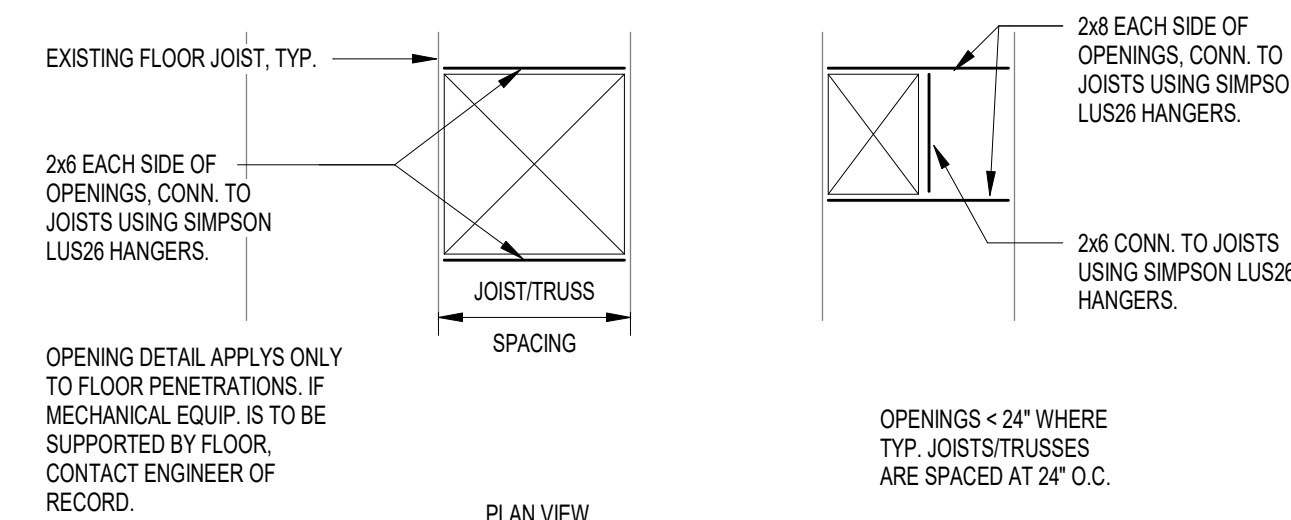
NOTES:  
1. REFER TO ARCH AND MECH DRAWINGS FOR LOCATION AND SIZE OF OPENINGS FOR NON-BEARING MASONRY WALLS.  
2. PROVIDE MINIMUM 6" BEARING ON BRICK, SOLID OR GROUTED SOLID CONCRETE BLOCK, BUT NOT LESS THAN 1" OF BEARING PER FOOT OF SPAN OF BEARING PER FOOT OF SPAN.  
3. ALL ANGLES LONG LEG VERTICAL.

3 TYPICAL DETAIL MISCELLANEOUS BOND OR PRECAST MASONRY LINTEL SCHEDULE  
1/2" = 1'-0"

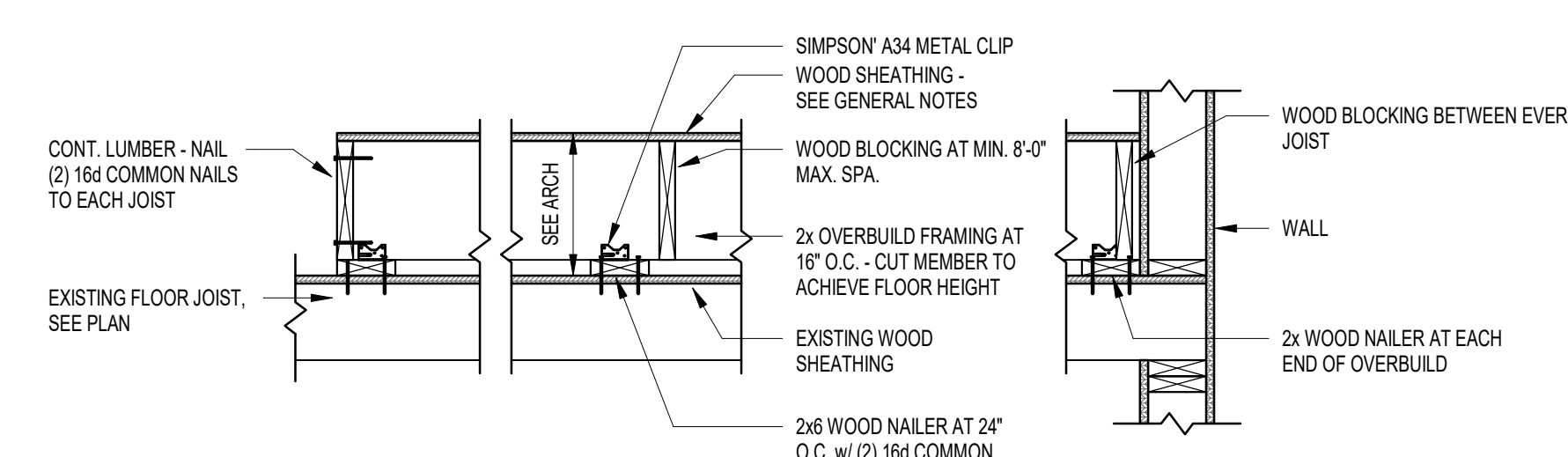


CONSTRUCTION SEQUENCE:  
1. SAWCUT KERF AT MORTAR JOINT ONE SIDE OF WALL.  
2. INSERT HORIZONTAL LEG OF ANGLE.  
3. SAWCUT KERF AT MORTAR JOINT ON OPPOSITE SIDE OF WALL.  
4. INSERT HORIZONTAL LEG OF SECOND ANGLE.  
5. INSTALL THRU BOLTS.  
6. DEMO WALL BELOW.  
7. INSTALL COVER PLATE.

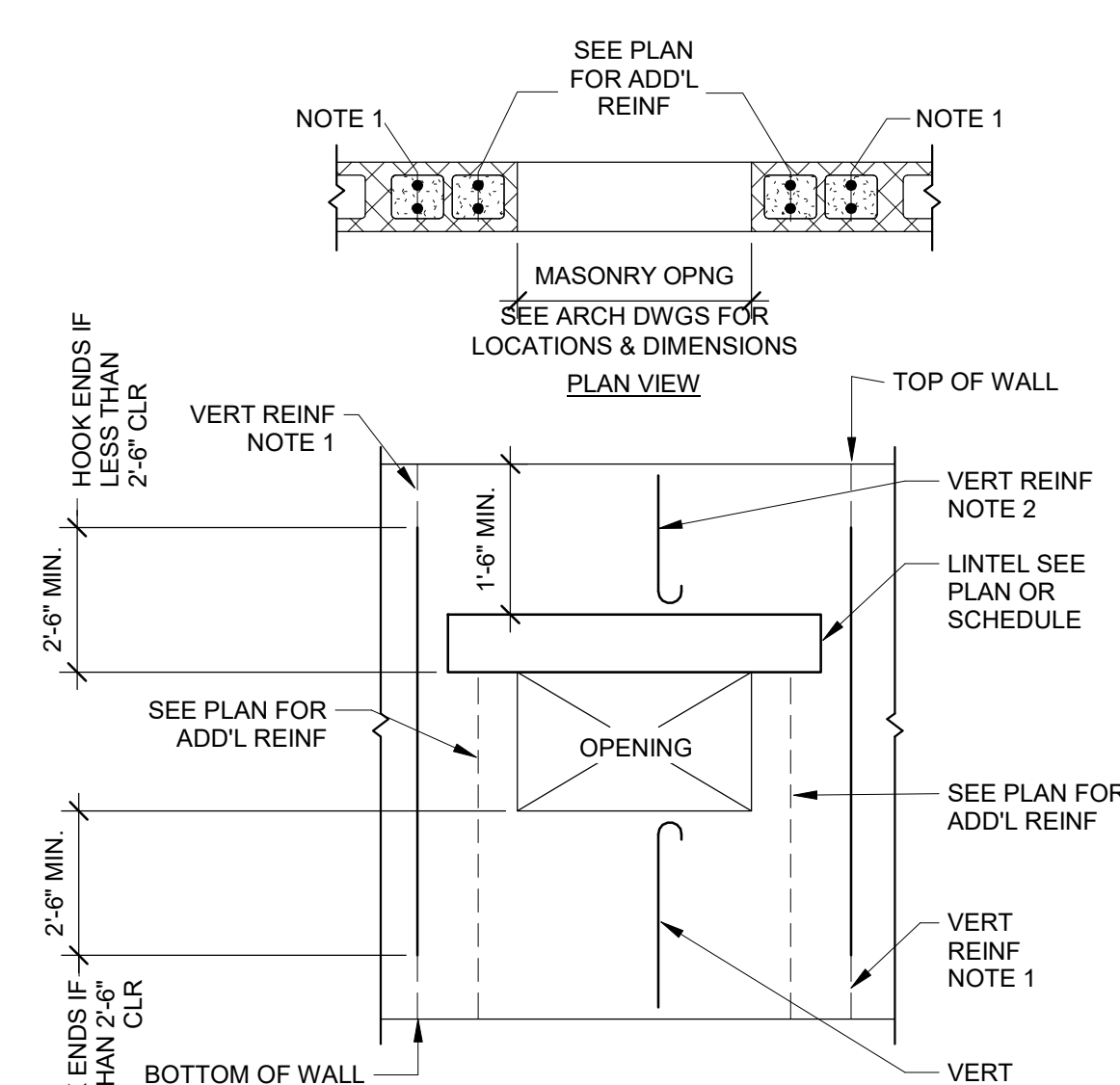
6 TYPICAL NEW OPENING IN EXISTING MASONRY WALL DETAIL  
3/4" = 1'-0"



5 TYPICAL FRAMING AT FLOOR OPENINGS  
3/4" = 1'-0"

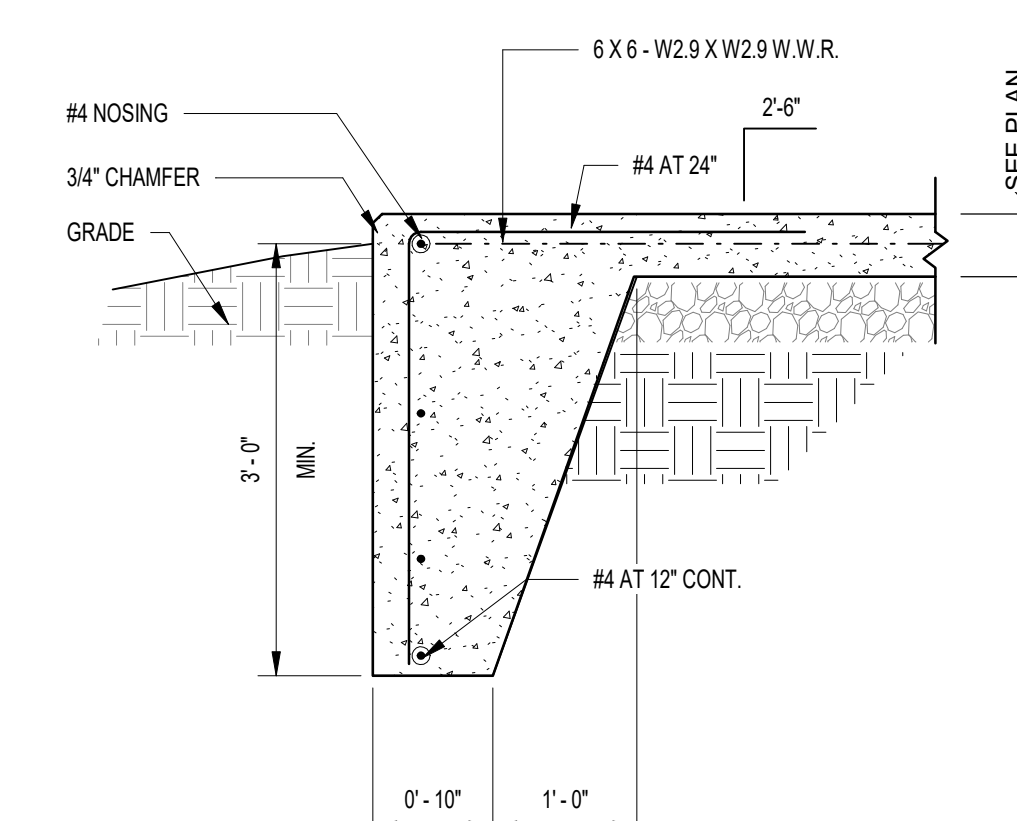


4 FLOOR OVERBUILD DETAIL  
3/4" = 1'-0"



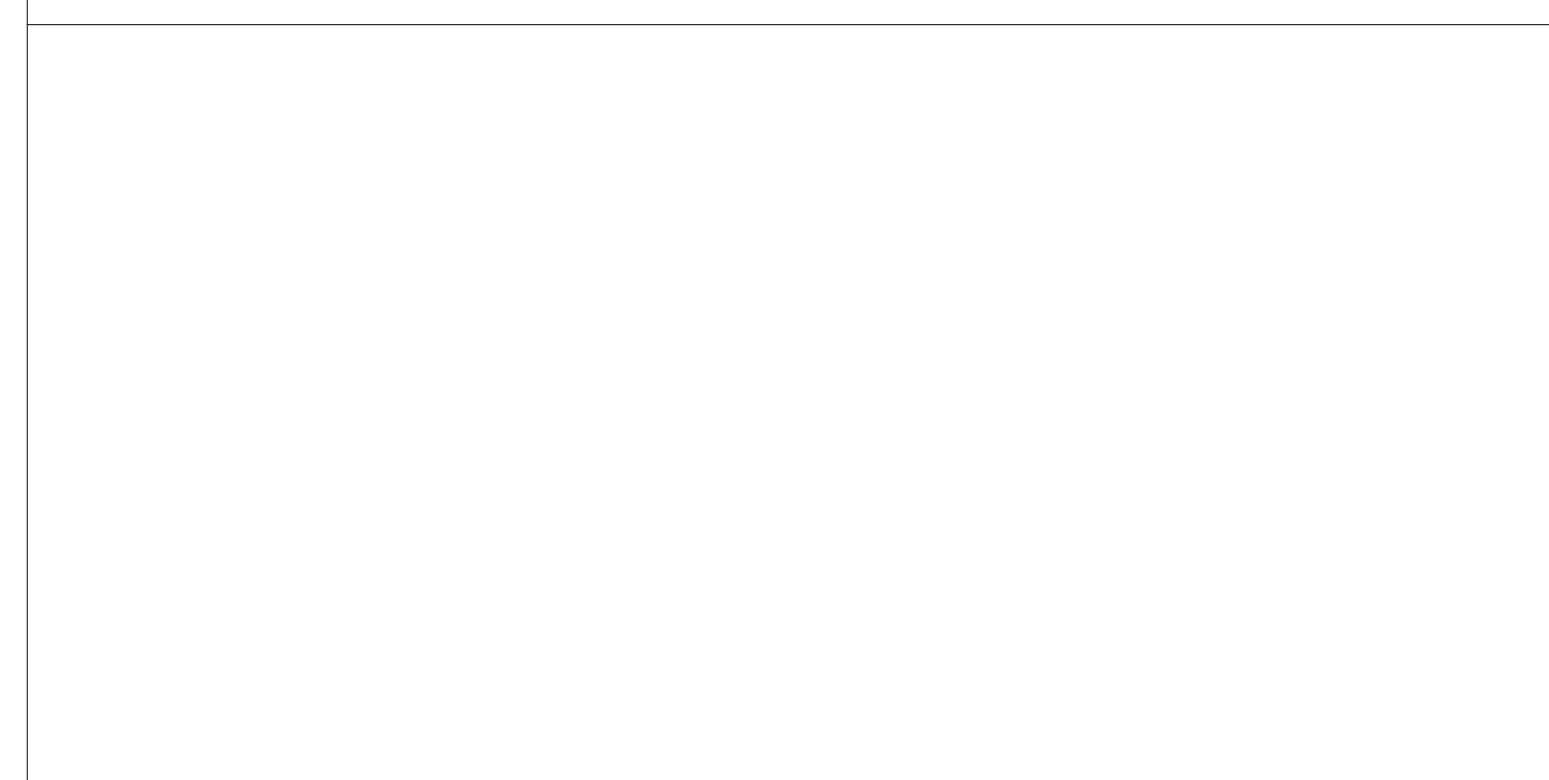
NOTES:  
1. REINFORCED WALL: ALL INTERRUPTED VERT. BARS SHALL BE COMPENSATED FOR BY ADDITIONAL BARS ON EACH SIDE OF OPENING FULL HEIGHT OF WALL. TOTAL AREA OF ADD'L BARS SHALL EQUAL AREA OF INTERRUPTED BARS. PROVIDE MIN (2)#5 EACH SIDE OF OPENING.  
2. REINFORCED WALL: SIZE & SPACING OF REINF INDICATED ON PLAN. PROVIDE HOOKED ENDS ON ALL DISCONTINUOUS BARS.

2 MASONRY WALL OPENING  
1/2" = 1'-0"



1 DOWN-TURNED SLAB DETAIL  
3/4" = 1'-0"

STAMP AREA



DRAWING TITLE  
**TYPICAL DETAILS**

PROJECT NO. **21070** DRAWING NO. **S302-R.2**

DATE: 08/28/23  
SCALE: As indicated  
DRAWN BY: W20  
CHECKED BY: PBU

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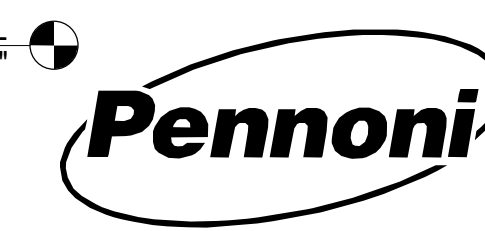
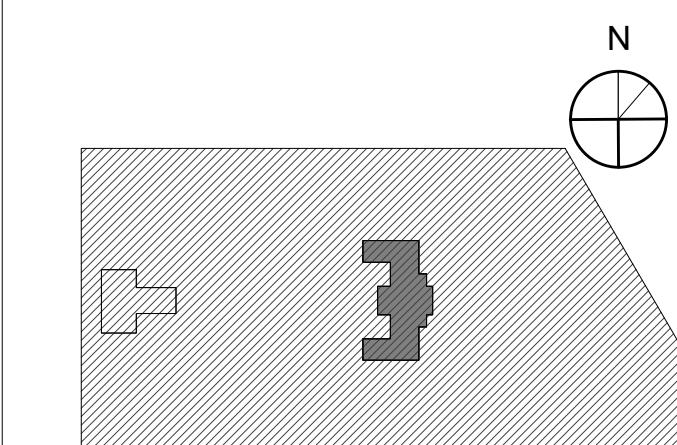
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KINGSSESSING RECREATION  
CENTER BUILDING AND SITE  
IMPROVEMENTS - PACKAGE 2

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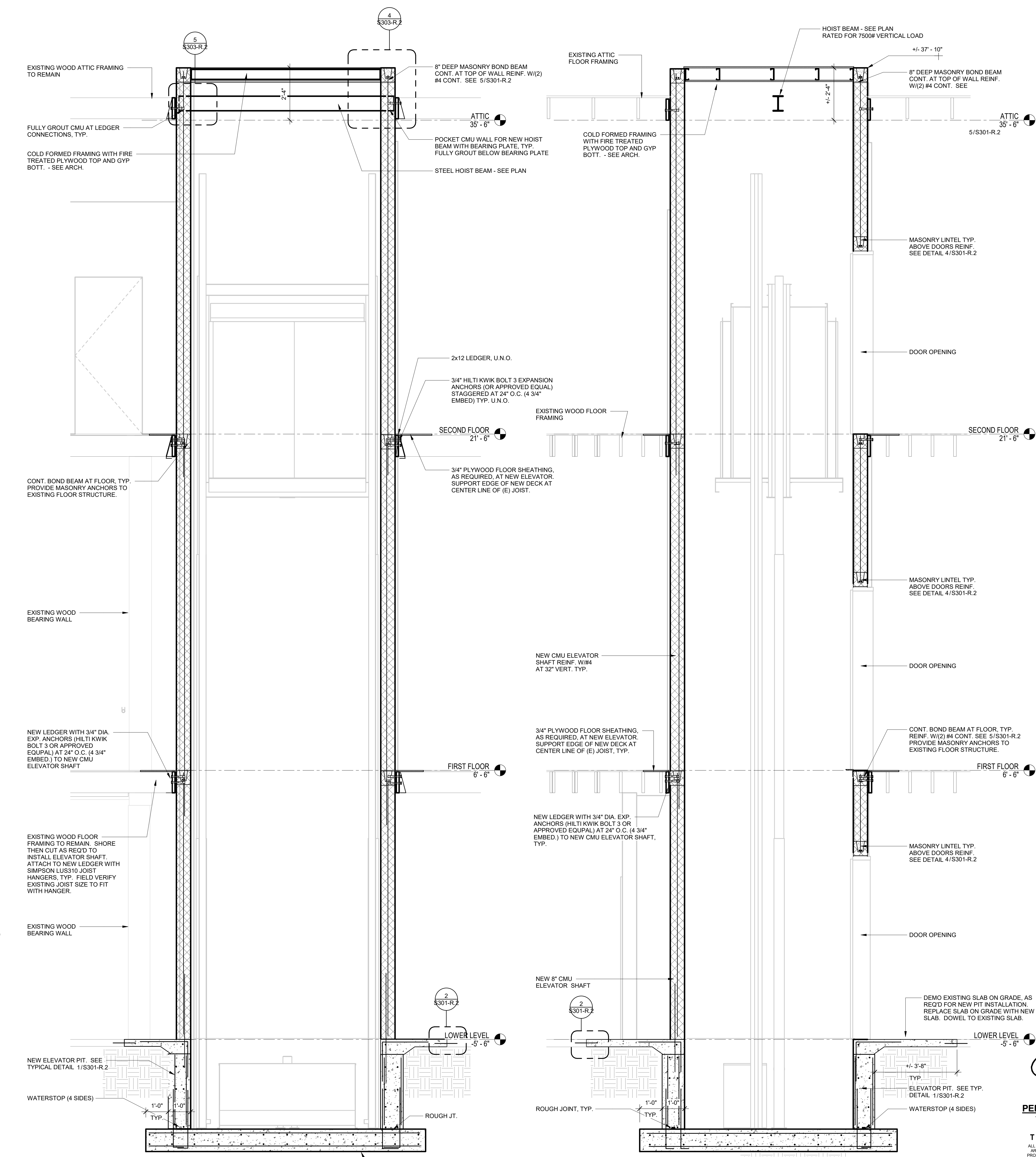
PROJECT NO.	DRAWING NO.
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21070

**S303-R.2**

DATE	SCALE	DRAWN BY	CHECKED BY
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DATE: 08/28/23  
SCALE: As indicated  
DRAWN BY: W20  
CHECKED BY: PBU



SECTION AT ELEVATOR  
1/2" = 1'-0"

SECTION AT ELEVATOR  
1/2" = 1'-0"

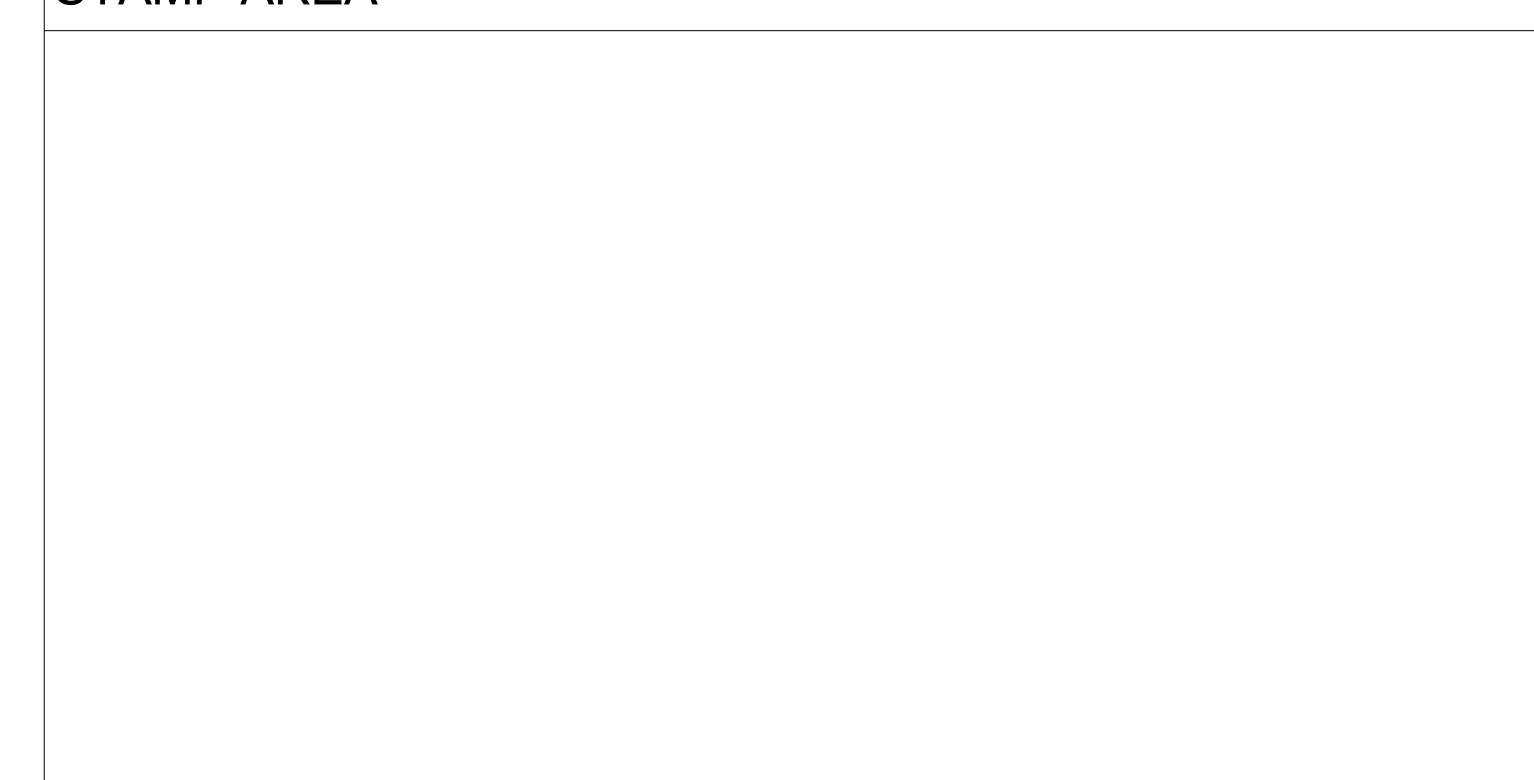
SECTION AT FLOOR INFILL  
3/4" = 1'-0"

HOIST BEAM BEARING PERPENDICULAR TO MASONRY WALL  
3/4" = 1'-0"

SECTION AT ELEVATOR SHAFT CEILING  
1" = 1'-0"

SECTION  
1/2" = 1'-0"

STAMP AREA





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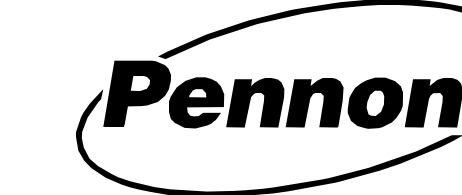
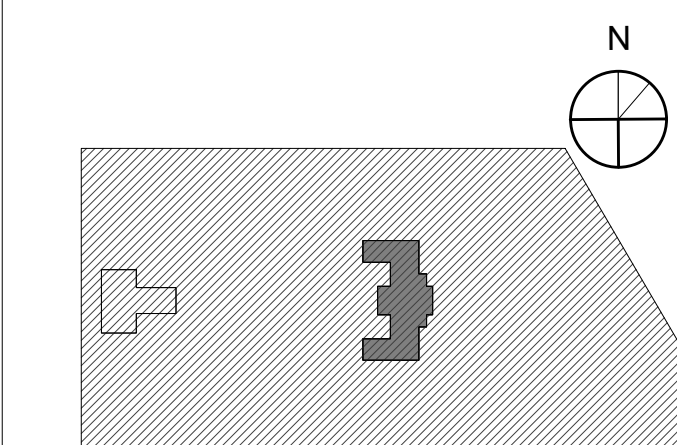
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SECTIONS

PROJECT NO. 21070

DATE 08/28/23

SCALE As indicated

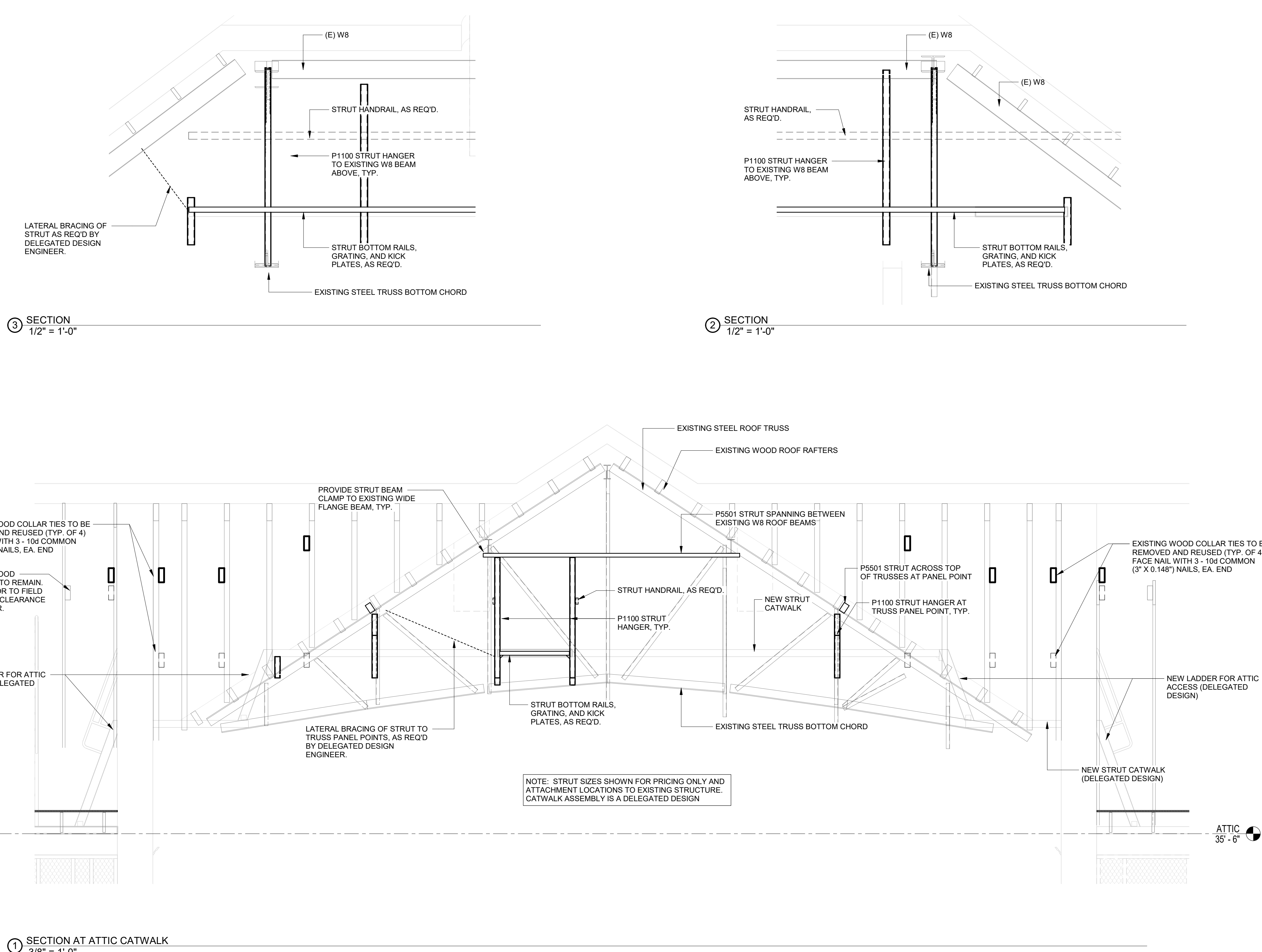
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**S-304-R.2**



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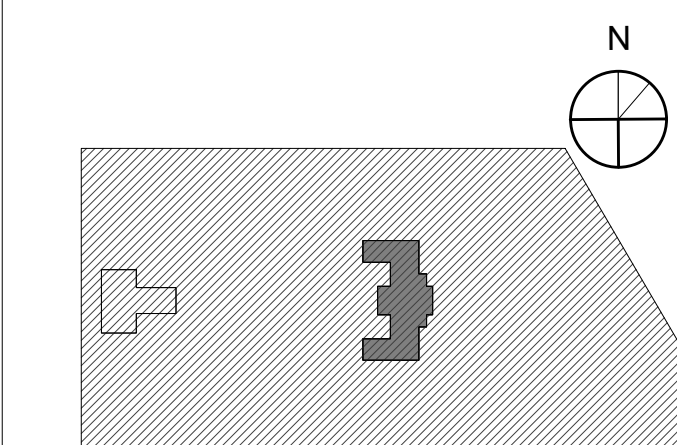
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PROJECT TITLE  
KINGSESSING RECREATION CENTER BUILDING AND SITE IMPROVEMENTS - PACKAGE 2

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DRAWING TITLE  
SECTIONS

PROJECT NO.  
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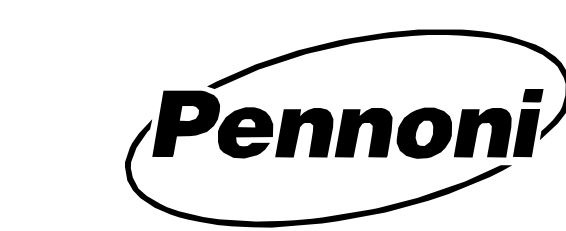
DATE  
08/28/23

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W20

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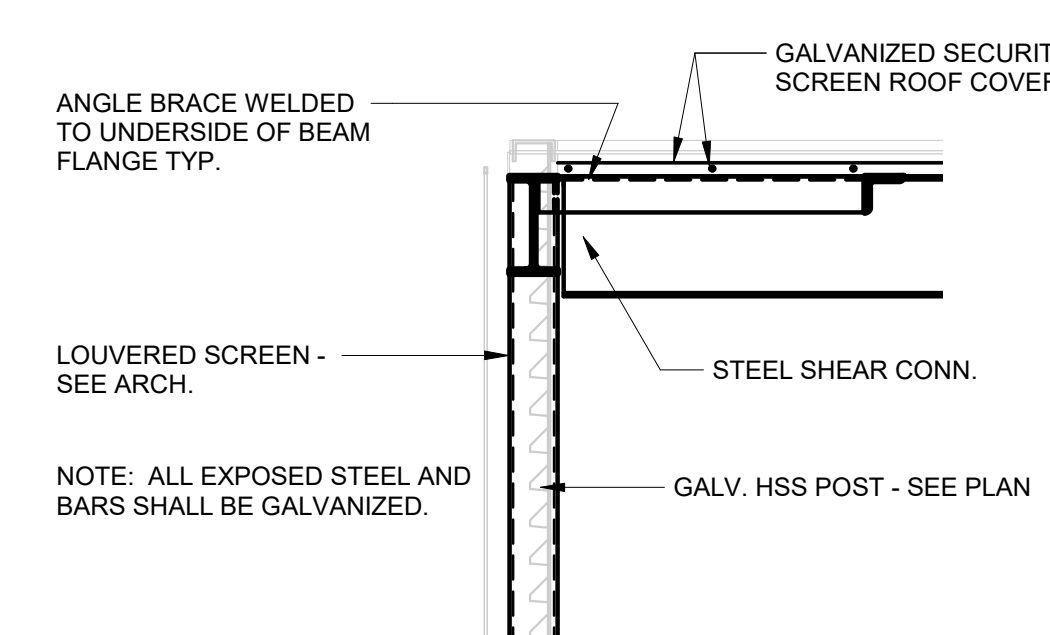
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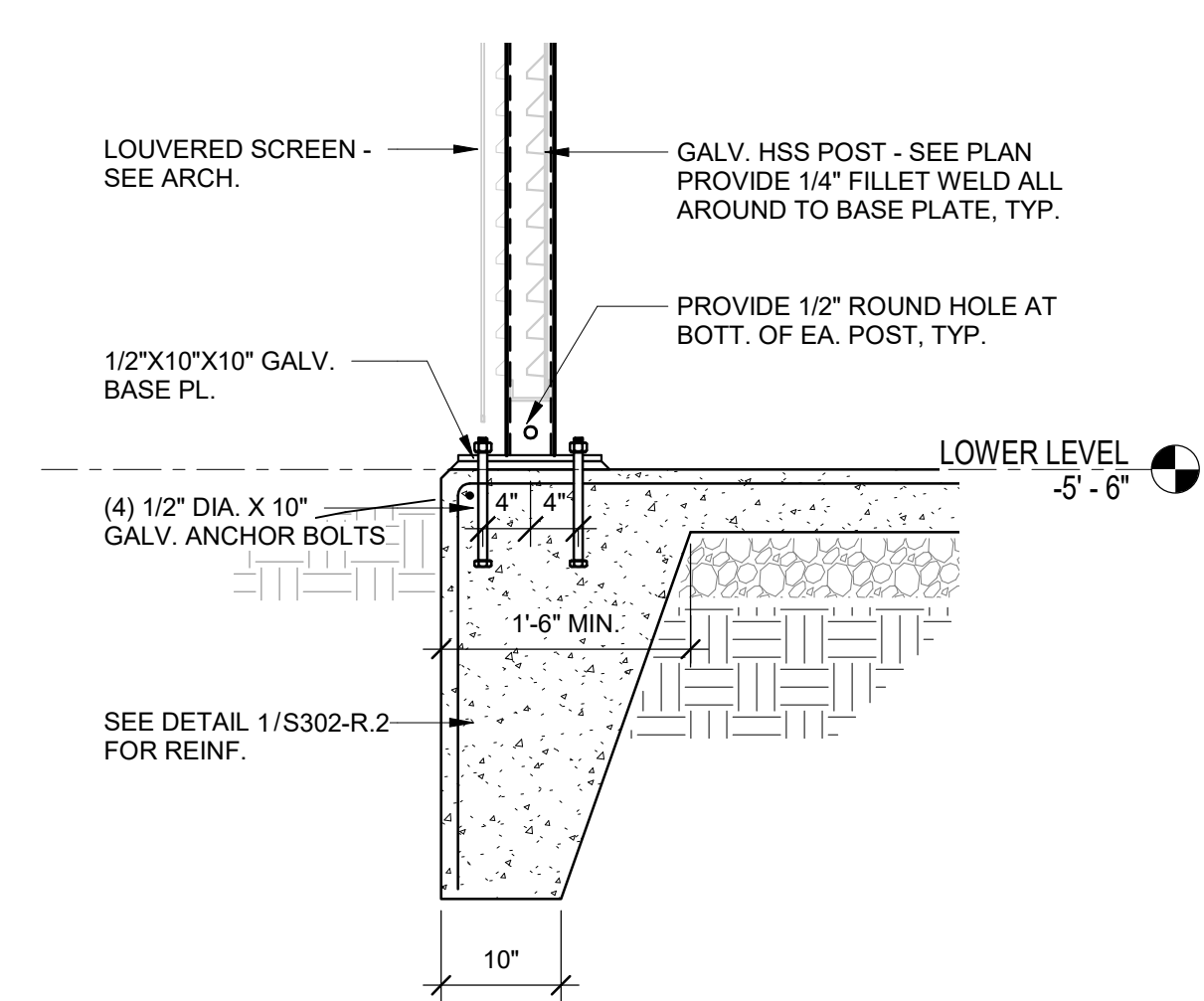
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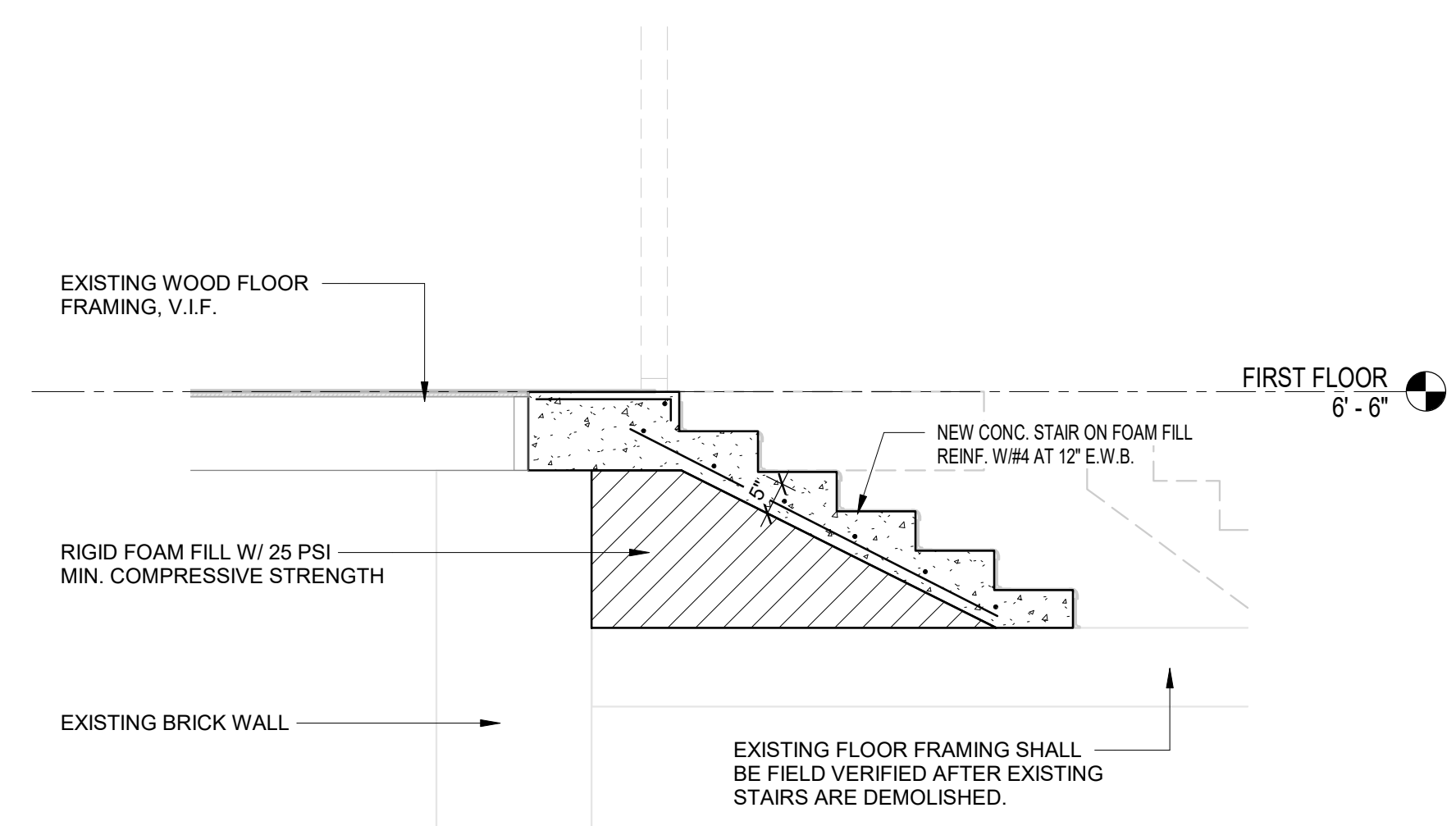
PACKAGE 2 - IFB  
NOT FOR CONSTRUCTION  
08/28/23



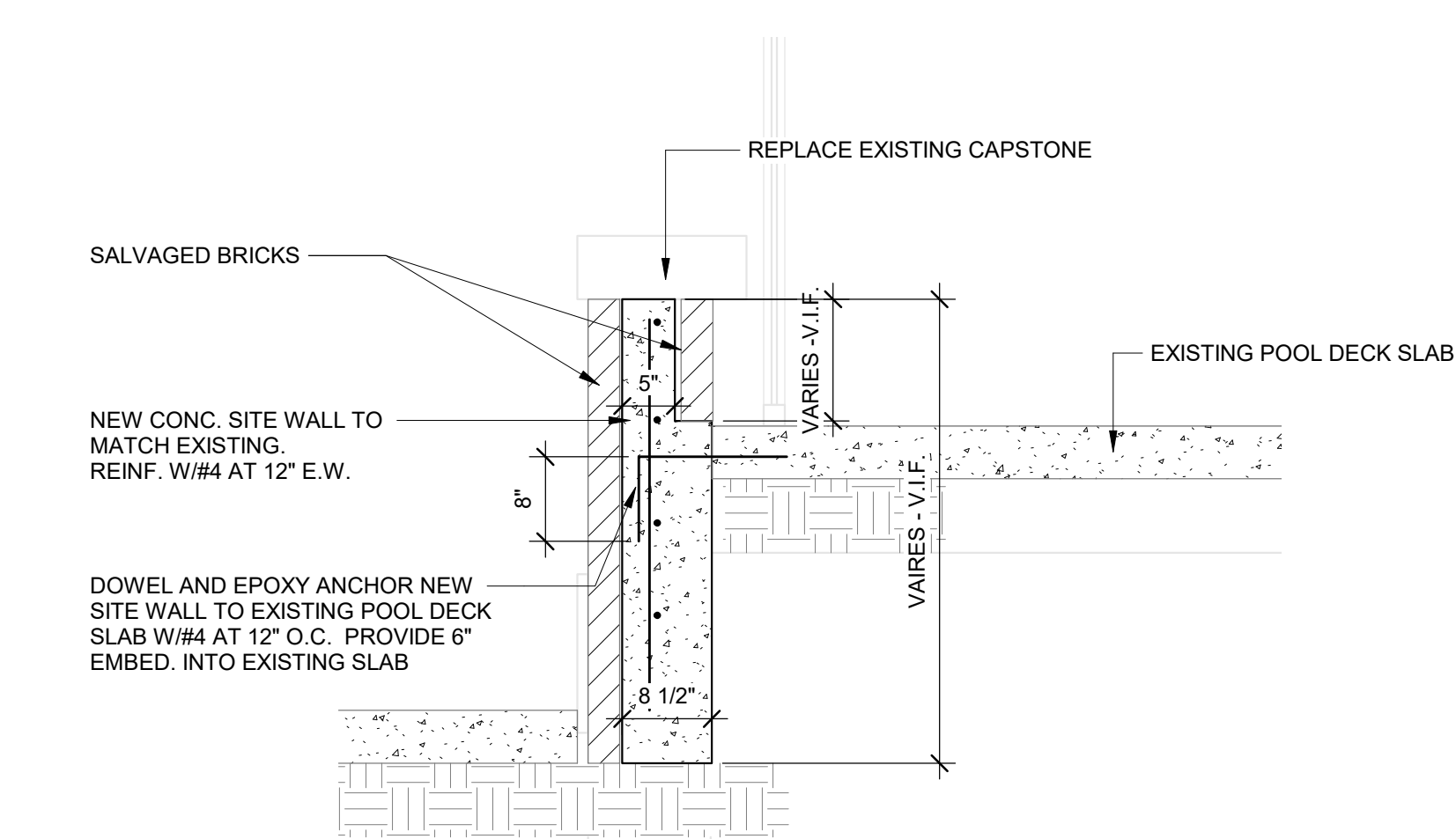
5 SECTION  
3/4" = 1'-0"



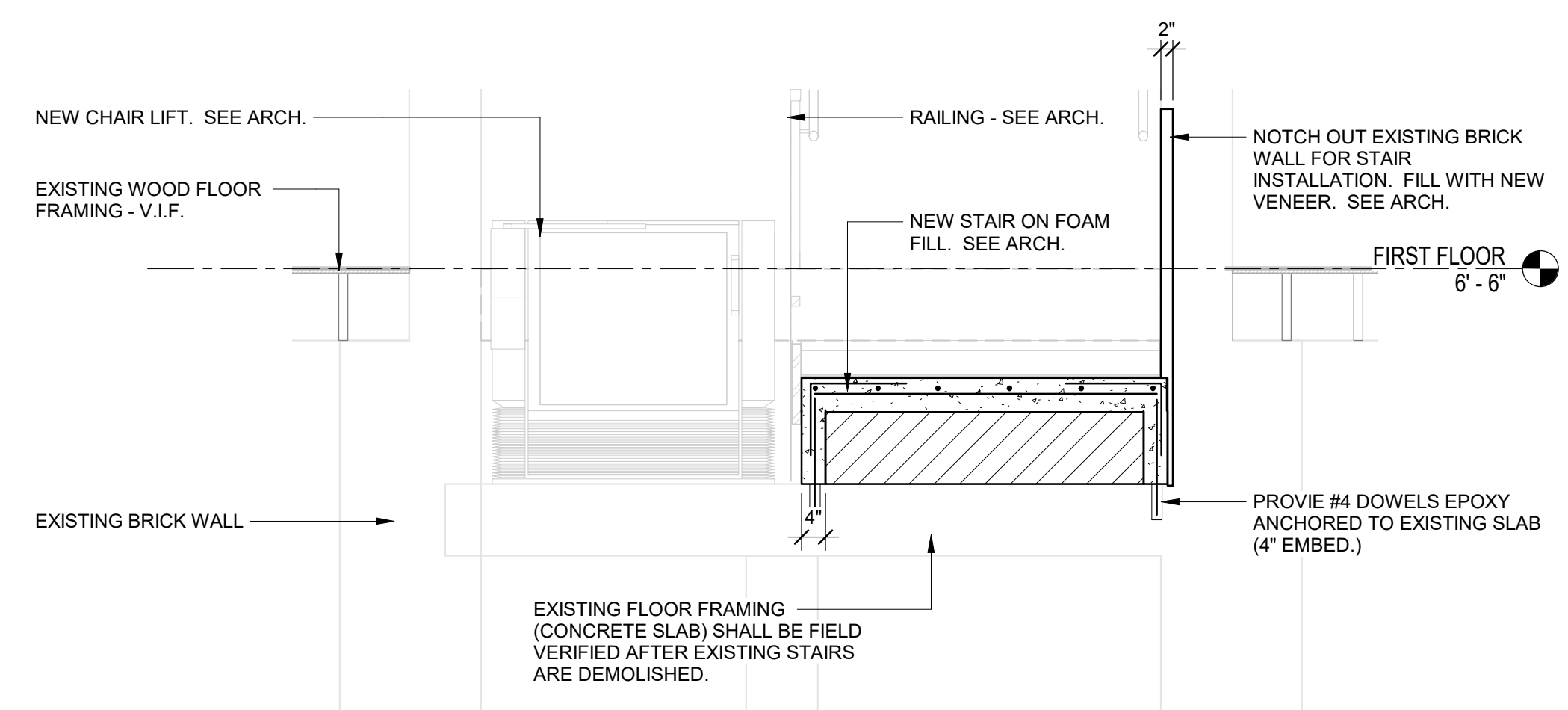
4 SECTION  
3/4" = 1'-0"



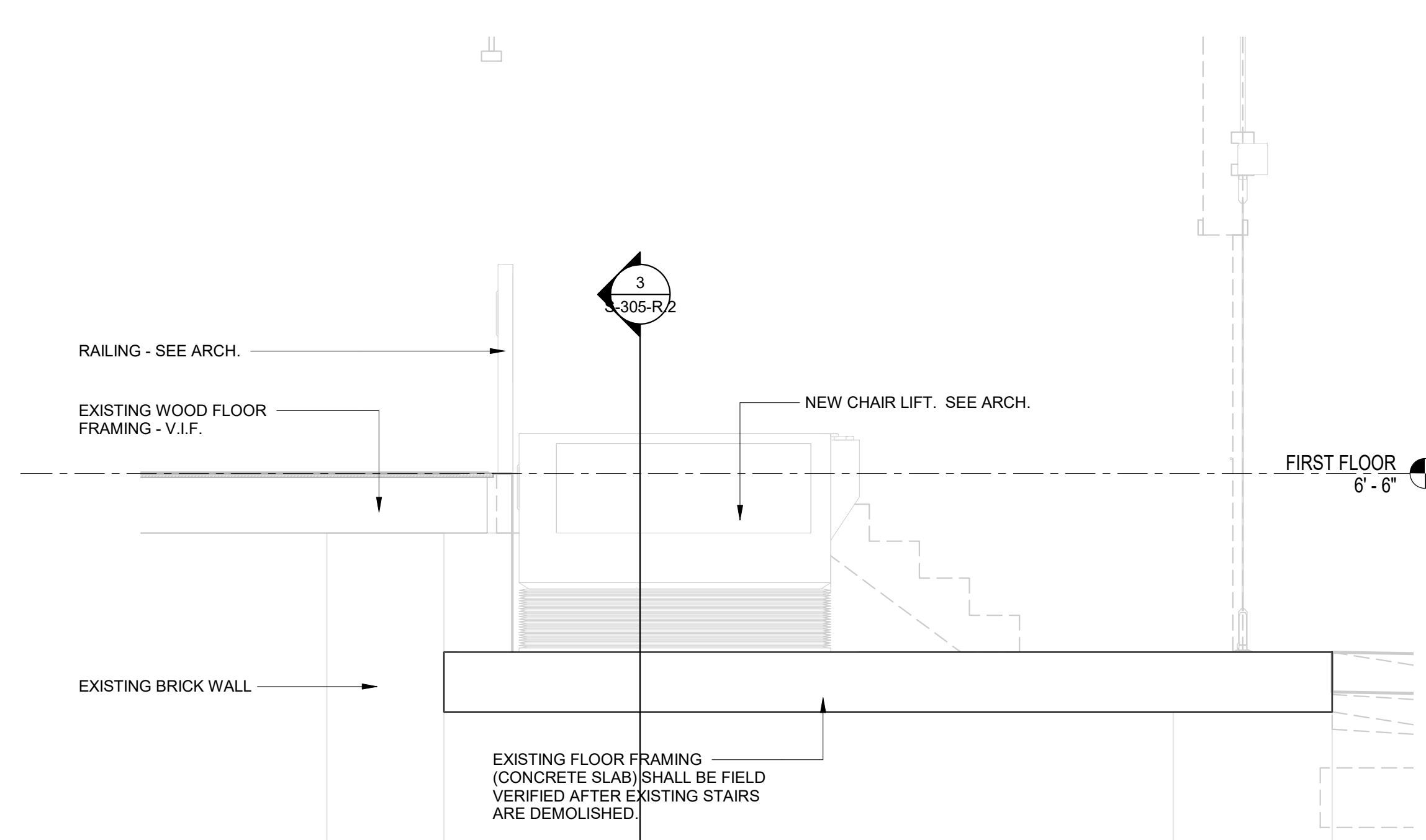
2 SECTION  
1/2" = 1'-0"



6 SECTION AT SITE WALL  
3/4" = 1'-0"



3 SECTION  
1/2" = 1'-0"



1 SECTION  
1/2" = 1'-0"

STAMP AREA